

#### 2018 Water System Plan, Tacoma Water

### Chapter 6, Appendix I: Distribution System Analysis

Latest Revision: February 28, 2018 (Created February 28, 2018) Author: M. Hubbard

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Chapter 6, Appendix I: Distribution System Analysis

## Chapter I1. Modeled Demands

#### Projected Demands and Factors for TW Hydraulic Model Analysis

Factor to multiply FROM 2017 A	DD TO:
2027 pADD	0.863
2037 pADD	0.797
2017 pMDD	2.548
2027 pMDD	2.550
2037 pMDD	2.553
Factor to multiply FROM ADD T	0:
PHD (residential)	3.11
PHD (commerical)	1.16
Assumed ERU Value:	

Figures from "ADD by Rate Cat - Consev" and "MDD - WSP.xlsx, MDD" for Maximum Day using the Maximum Monthly Demand value per Year

		MGD	
			Calculated
Year	Total	Simpson	Remainder
2017 Projected ADD	48.18	16.07	32.11
MDD	97.90	16.07	81.83
Factor	2.032		2.548
2027 Projected ADD	43.80	16.07	27.73
MDD	97.97	16.07	81.90
Factor	2.237		2.954
2037 Projected ADD	41.66	16.07	25.59
MDD	98.06	16.07	81.99
Factor	2.354		3.204

Adjust actual 2016 ADD Demand to 2017 projected

			Calculated
	Total	Simpson	Remainder
2016 Actual ADD	53.64	15.51	38.13
2017 Projected ADD	48.18	16.07	32.11
Factor	0.898	1.036	0.842

Note 53.64 was total average production for 2016 on "Monthly and Annual Production" and does not include 9.43 MGD for Partners Calculated Remainder includes:

Wholesale accounts other than Partners (Fife, etc) Residential

Commercial

Partners flow to remain flat at 9.43 MGD

PHD demand factors were calculated based on data from 2009-2016 Residential PHD calculations assumed outputs from Indian Hills Pump Station, pumps #1 and #2, is representative of residential demand Commerical PHD calculations assumed output Hood Street's 24" outlet, serving the downtown core, is representative of commerical demand

Lowercase "p" denotes a projected demand



Chapter 6, Appendix I: Distribution System Analysis

## **Chapter I2. Peak Hour Demand Assessment**

#### Tacoma Water Distribution System Analysis Revised: 1/29/2018

Westgate / Fletcher Service 538

University Place 531

Sunrise Terrace 519

South East Tacoma 520

38.1

44.5

57.1

29.2

0

0

0

2

0

0

0

0.3

0

0

0

3

0

0

0

0

0

0

0

0

Peak Hour Demand Assessment Nodes Below Demand Calculated Nodes Below Demand Calculated Calculated Total Nodes Pressure Zone and HGL Elevation Pressure, No Overall Demand in ERU's in 30 psi, Non-Below 30 psi **ERU's Below** 27 psi, Non-Below 27 psi ERU's Below **Recommended Acti** Zero Deman Assessment Zero Demand (gpm) 30 psi Zero Demand (gpm) 27 psi Zone (gpm) Nodes (psi) Bonney Lake 1010 57.6 0 0 0 0 0 0 178 86 673 Adequate Bonney Lake 950 73.6 0 0 0 0 0 0 778 136 1,061 Adequate \*Field verification re shows 29.42 psi, wit Cumberland 931 29.4 1 8 0 0 0 50 8 64 Adequate\* psi), connection bet 1 12" main just south c Bonney Lake 860 58.1 0 0 0 0 0 0 30 21 162 Adequate 43.8 0 1,405 10,981 Prairie Ridge 810 0 0 0 0 0 2,830 Adequate 35.0 3,094 1,805 McMillin 706 0 0 0 0 0 0 14,110 Adequate Fennel Creek 705 51.7 0 0 0 0 0 0 205 102 801 Adequate South Summit High 669 70.4 0 0 0 0 0 0 885 401 3,135 Adequate Indian Hill 649 40.3 336 2,629 0 0 0 641 0 0 0 Adequate 80th Avenue E 626 53.7 0 0 0 0 0 0 26 24 191 Adequate Alder Lane 626 70.9 0 0 0 8 62 0 0 0 14 Adequate Highland 621 30.1 0 0 0 0 0 0 523 269 2,103 Adequate 50.9 41 Frederickson 588 0 0 0 0 0 0 59 319 Adequate South East Tacoma 581 41.2 0 0 0 0 0 0 11 2 15 Adequate 41.9 253 113 Fletcher Heights 581 0 0 0 0 0 0 886 Adequate Woodland 581 55.2 0 0 0 0 0 357 210 1,642 Adequate 0 South Hill 581 40.0 0 0 0 0 0 1,327 537 4,201 0 Adequate \*Field verification re shows 27.3 psi, with 27.3 1 9 0 0 0 5,597 24,273 Canyon 581 1 3,105 Adequate\* psi), intersection of Bonney Lake 581 N/A 0 0 0 0 0 0 4 0 0 Adequate Park Royal 556 47.5 0 0 0 0 0 0 207 118 925 Adequate \*Field verification re shows 29.2 psi, with Northeast Tacoma 549 29.2 2 1 6 0 0 0 3,393 1,785 13,952 Adequate\* psi), intersection of

1,597

2,307

1,158

86

1,186

1,302

616

24

9,272

10,180

4,817

190

Adequate

Adequate

Adequate

Adequate\*

0

0

0

0

Recommended Action/ Comment	2017 Assessment	2027 Assessment	2037 Assessment
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
*Field verification required (model shows 29.42 psi, within 10% of 30 psi), connection between 8" and 12" main just south of Cumberland Tank connection	Adequate*	Adequate*	Adequate*
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
*Field verification required (model shows 27.3 psi, within 10% of 30 psi), intersection of E 53rd Street and E J Street	Adequate*	Adequate*	Adequate*
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
*Field verification required (model shows 29.2 psi, within 10% of 30 psi), intersection of Norpoint Way and Northshore Highway	Adequate*	Adequate*	Adequate*
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
None	Adequate	Adequate	Adequate
*Field verification required (model shows 29.2 psi, within 10% of 30 psi), intersection of 120th St E and Military Rd E	Adequate*	Adequate*	Adequate*

	Peak Hour Demand Assessment														
Pressure Zone and HGL Elevation (ft)	Minimum Pressure, Non- Zero Demand Nodes (psi)	Nodes Below 30 psi, Non- Zero Demand	Demand Below 30 psi (gpm)	Calculated ERU's Below 30 psi	Nodes Below 27 psi, Non- Zero Demand	Demand Below 27 psi (gpm)	Calculated ERU's Below 27 psi	Total Nodes in Zone	Total Demand in Zone (gpm)	Calculated ERU's in Zone	Overall Assessment	Recommended Action/ Comment	2017 Assessment	2027 Assessment	2037 Assessment
Beverly Heights 486	47.2	0	0	0	0	0	0	114	52	409	Adequate	None	Adequate	Adequate	Adequate
High Service 478	20.5	26	25	198	7	4	28	22,373	13,393	104,690	Adequate*	*Project in progress to address 7 nodes less than 27psi; field verification required for other 26 locations	Adequate*	Adequate*	Adequate*
Middle Service 446	32.0	0	0	0	0	0	0	1,184	849	6,639	Adequate	None	Adequate	Adequate	Adequate
North End Service 446	28.3	3	6	43	0	0	0	1,426	1,001	7,824	Adequate*	*Field verification required (model shows 28.3 psi, within 10% of 30 psi), intersection of N Bennet Street and N 35th Street	Adequate*	Adequate*	Adequate*
Harbor View 426	59.5	0	0	0	0	0	0	40	28	216	Adequate	None	Adequate	Adequate	Adequate
Woodland 426	54.1	0	0	0	0	0	0	81	60	471	Adequate	None	Adequate	Adequate	Adequate
Dash High Point 411	39.4	0	0	0	0	0	0	39	27	209	Adequate	None	Adequate	Adequate	Adequate
Fife Heights Low 411	48.8	0	0	0	0	0	0	114	63	494	Adequate	None	Adequate	Adequate	Adequate
Twin Lakes 411	39.0	0	0	0	0	0	0	956	470	3,676	Adequate	None	Adequate	Adequate	Adequate
Overlook 370	51.7	0	0	0	0	0	0	87	89	695	Adequate	None	Adequate	Adequate	Adequate
Grandview 351	38.7	0	0	0	0	0	0	168	67	526	Adequate	None	Adequate	Adequate	Adequate
Salmon Beach North 350	N/A	0	0	0	0	0	0	11	0	0	Adequate	None	Adequate	Adequate	Adequate
Browns / Dash Points 346	34.9	0	0	0	0	0	0	386	125	979	Adequate	None	Adequate	Adequate	Adequate
North East Tacoma 346	37.8	0	0	0	0	0	0	550	143	1,117	Adequate	None	Adequate	Adequate	Adequate
Old Town 346	46.7	0	0	0	0	0	0	69	50	389	Adequate	None	Adequate	Adequate	Adequate
Portland Avenue 346	30.9	0	0	0	0	0	0	424	219	1,709	Adequate	None	Adequate	Adequate	Adequate
Narrows 328	39.4	0	0	0	0	0	0	640	352	2,755	Adequate	None	Adequate	Adequate	Adequate
High Cedars 316	71.6	0	0	0	0	0	0	423	215	1,682	Adequate	None	Adequate	Adequate	Adequate
Chambers Bay 290	107.6	0	0	0	0	0	0	17	0	3	Adequate	None	Adequate	Adequate	Adequate
Low Service 251	30.9	0	0	0	0	0	0	3,361	14,649	114,509	Adequate	None	Adequate	Adequate	Adequate
Dash Low Point 226	49.3	0	0	0	0	0	0	35	15	119	Adequate	None	Adequate	Adequate	Adequate
Hyada 226	42.1	0	0	0	0	0	0	152	104	813	Adequate	None	Adequate	Adequate	Adequate
Titlow 226	45.5	0	0	0	0	0	0	122	94	733	Adequate	None	Adequate	Adequate	Adequate
Day Island 202	65.8	0	0	0	0	0	0	53	39	306	Adequate	None	Adequate	Adequate	Adequate
Lakota Beach 186	49.2	0	0	0	0	0	0	62	30	231	Adequate	None	Adequate	Adequate	Adequate
Sunset Beach 155	61.3	0	0	0	0	0	0	6	4	35	Adequate	None	Adequate	Adequate	Adequate



Chapter 6, Appendix I: Distribution System Analysis

## Chapter I3. Maximum Day Demand + Fire Flow Assessment

#### Tacoma Water Distribution System Analysis 1/29/2018

Revised:

	Maximum Day Demand + Fire Flow Assessment													
Pressure Zone and HGL Elevation (ft)	Maximum Available FF (gpm)	Minimum Available FF (gpm)	Basic Land Use Type	Tacoma Water Land Use Planning Level FF Target (gpm)	Juristiction Land Use Type	Juristiction	Juristiction Required FF (gpm)	Nodes Below 20 psi During FF	Total Nodes in Zone	Overall Assessment	Recommended Action/ Comment	2017 Assessment	2027 Assessment	2037 Assessment
Bonney Lake 1010	1,791	1,306	Residential	1,500	Employment Based Planned Community	Pierce County	750	0	178	Adequate	None	Adequate	Adequate	Adequate
Bonney Lake 950	1,966	1,355	Residential	1,500	Employment Based Planned Community	Pierce County	750	0	778	Adequate	None	Adequate	Adequate	Adequate
Cumberland 931	1,741	1,057	Residential	1,500	Rural Area	King	1,000	0	50	Adequate	None	Adequate	Adequate	Adequate
Bonney Lake 860	1,405	1,313	Residential	1,500	Employment Based Planned Community	Pierce County	750	0	30	Adequate	None	Adequate	Adequate	Adequate
Prairie Ridge 810	19,466	1,081	City	3,500	Bonney Lake	Pierce	1,500	0	2,830	Adequate	Multiple land use types within zone, fire flow requirements are met for each type	Adequate	Adequate	Adequate
McMillin 706	104,580	868	Commercial	3,500	Community Center	Pierce County	1,500	3	3,094	Additional Analysis Required	Multiple land use types within zone, review 6" AC main on 132nd Ave E for upsizing	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Fennel Creek 705	1,504	1,500	Residential	1,500	Rural 5	Pierce County	750	0	205	Adequate	None	Adequate	Adequate	Adequate
South Summit High 669	16,552	635	Commercial	3,500	Community Employment	Pierce	1,500	17	885	Additional Analysis Required	Select mains should be assessed for replacement, it appears 4" mains are reducing available fire flow	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Indian Hill 649	3,292	377	Residential	1,500	Single Family Residental	Tacoma	1,000	142	641	Additional Analysis Required	Node located at intersection of Tower Ln NE and Tower Dr NE is limiting FF due to elevation, review system for adjusting zone pressure upwards	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
80th Avenue E 626	2,118	1,285	Residential	1,500	Moderate Density Single Family	Pierce	1,000	0	26	Adequate	None	Adequate	Adequate	Adequate
Alder Lane 626	1,012	368	Residential	1,500	Moderate Density Single Family	Pierce County	750	2	14	Additional Analysis Required	Mains should be assessed for replacement, it appears 4" mains are reducing available fire flow	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Highland 621	2,621	625	Residential High Density	3,500	High Density Single Family	Pierce County	1,500	23	523	Additional Analysis Required	Review pressure zone and pump station settings for potentially adjusting pressure upwards	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required

	Maximum Day Demand + Fire Flow Assessment													
Pressure Zone and HGL Elevation (ft)	Maximum Available FF (gpm)	Minimum Available FF (gpm)	Basic Land Use Type	Tacoma Water Land Use Planning Level FF Target (gpm)	Juristiction Land Use Type	Juristiction	Juristiction Required FF (gpm)	Nodes Below 20 psi During FF	Total Nodes in Zone	Overall Assessment	Recommended Action/ Comment	2017 Assessment	2027 Assessment	2037 Assessment
Frederickson 588	4,006	2,256	Commercial	3,500	Employment Center	Pierce	1,500	0	59	Adequate	None	Adequate	Adequate	Adequate
South East Tacoma 581	246	124	Commercial	3,500	Community Employment	Pierce	1,500	11	11	Additional Analysis Required	Nodes are served directly off of 58" transmission main with 2" galvanized steel manifolds, review for eliminating 2" manifolds	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Fletcher Heights 581	10,148	861	Commercial	3,500	Major Institutional Campus	Tacoma	1,500	7	253	Additional Analysis Required	Review for adjusting pressure zone boundary on western edge of zone, 6' main appears to limit flow at dead ends	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Woodland 581	11,407	715	Residential	1,500	Moderate Density Single Family	Pierce County	750	3	357	Additional Analysis Required	Review 4" and 6" AC mains for upsizing or looping, deficient node is located at dead end	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
South Hill 581	55,418	592	Commercial	3,500	Community Center	Pierce	1,500	4	1,327	Additional Analysis Required	Mains should be assessed for replacement, it appears 4" mains are reducing available fire flow along 72nd Ave E and 73rd Ave E	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Canyon 581	51,998	195	Mixed Use	3,500	Mixed Use District	Pierce County	1,500	57	5,597	Additional Analysis Required	Review 4" and 6" AC/Plastic aquired mains for upsizing or looping, deficient nodes generally located at dead ends	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Bonney Lake 581	37,774	37,225	Residential	1,500	Rural 10	Pierce	750	0	4	Adequate	None	Adequate	Adequate	Adequate
Park Royal 556	2,562	814	City	3,500	University Place	Pierce County	1,500	22	207	Additional Analysis Required	Review 6" AC aquired mains for upsizing or looping throughout zone, deficient nodes generally located at dead ends of cul-de- sacs	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required

	Maximum Day Demand + Fire Flow Assessment													
Pressure Zone and HGL Elevation (ft)	Maximum Available FF (gpm)	Minimum Available FF (gpm)	Basic Land Use Type	Tacoma Water Land Use Planning Level FF Target (gpm)	Juristiction Land Use Type	Juristiction	Juristiction Required FF (gpm)	Nodes Below 20 psi During FF	Total Nodes in Zone	Overall Assessment	Recommended Action/ Comment	2017 Assessment	2027 Assessment	2037 Assessment
Northeast Tacoma 549	17,816	888	Commercial	5,000	General Commercial	King County	1,000	10	3,393	Additional Analysis Required	Multiple land use types within zone, fire flow requirements are met for each type; 6 nodes are deficient on edge of zone, review 6" and 4" mains for upsizing at these locations	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Westgate / Fletcher Service 538	8,198	316	Commercial	3,500	Major Institutional Campus	Tacoma	1,500	41	1,597	Additional Analysis Required	Nodes are generally deficient along dead ends east of N Stevens Ave, Review old 6" cast iron mains for upsizing at these locations	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
University Place 531	7,856	403	City	3,500	University Place	Pierce County	1,500	17	2,307	Additional Analysis Required	Review 6" mains for upsizing, particulalry near pressure zone boundaries	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
South East Tacoma 520	5,897	301	Mixed Use	3,500	Commercial Mixed Use District	Pierce County	1,500	16	1,158	Additional Analysis Required	Review 4" AC mains for upsizing, identified nodes are located at constrictions and dead end	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Sunrise Terrace 519	2,217	528	Residential	1,500	Moderate Density Single Family	Pierce	750	8	86	Additional Analysis Required	6" ductile iron main along 120th Street E appears to be undersized, review for upsizing or looping	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Beverly Heights 486	2,748	1,787	Residential	1,500	Single Family Residental	Tacoma	1,000	0	114	Adequate	None	Adequate	Adequate	Adequate
High Service 478	38,599	29	Commercial	5,000	General Commercial	Tacoma	1,500	457	22,373	Additional Analysis Required	Review 6" mains for upsizing, particulalry near pressure zone boundaries	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Middle Service 446	6,292	568	Mixed Use	3,500	Downtown Regional Growth Center	Tacoma	1,500	6	1,184	Additional Analysis Required	Multiple land use types within zone, fire flow requirements are met for each type; 4 nodes are deficient on edge of zone boundary with 478 on north Yakima Ave and N 8th Street, can likely be addressed by adjusting zone boundary	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required

						Max	kimum Day Dema	ind + Fire Flow As	sessment					
Pressure Zone and HGL Elevation (ft)	Maximum Available FF (gpm)	Minimum Available FF (gpm)	Basic Land Use Type	Tacoma Water Land Use Planning Level FF Target (gpm)	Juristiction Land Use Type	Juristiction	Juristiction Required FF (gpm)	Nodes Below 20 psi During FF	Total Nodes in Zone	Overall Assessment	Recommended Action/ Comment	2017 Assessment	2027 Assessment	2037 Assessment
North End Service 446	569	235	Residential High Density	3,500	Multi-Family (High Density)	Tacoma	1,500	546	1,426	Additional Analysis Required	Node located at intersection of N 35th and N Shirley is limiting FF due to elevation, review system for adjusting zone pressure upwards	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Harbor View 426	3,449	1,951	Residential	1,500	Single Family Residental	Tacoma	1,000	0	40	Adequate	None	Adequate	Adequate	Adequate
Woodland 426	3,449	736	Residential	1,500			750	7	81	Additional Analysis Required	6" AC main along Woodland Ave should be assessed for replacement, it appears to be reducing available fire flow	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Dash High Point 411	2,900	1,032	Residential	1,500	Single Family	Pierce	1,000	0	39	Adequate	None	Adequate	Adequate	Adequate
Fife Heights Low 411	1,875	929	Residential	1,500	Moderate Density Single Family	Pierce County	750	0	114	Adequate	None	Adequate	Adequate	Adequate
Twin Lakes 411	8,737	737	Residential	1,500	Undesignated	King County	1,000	2	956	Additional Analysis Required	Review 6" AC main for upsizing	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Overlook 370	4,013	2,104	Residential	1,500	Single Family Residential	Tacoma	1,000	0	87	Adequate	None	Adequate	Adequate	Adequate
Grandview 351	2,471	1,511	City	3,500	University Place	Pierce	1,500	0	168	Adequate	None	Adequate	Adequate	Adequate
Salmon Beach North 350	1,096	1,096	Park	1,000	Parks and Open Space	Tacoma	1,000	0	11	Adequate	None	Adequate	Adequate	Adequate
Browns / Dash Points 346	4,103	1,179	Commercial	2,000	Neighborhood Center	Pierce	1,500	0	386	Adequate	Multiple land use types within zone, fire flow requirements are met for each type	Adequate	Adequate	Adequate
North East Tacoma 346	2,667	592	Residential	1,500	Undesignated	King	1,000	8	550	Additional Analysis Required	Review 6" mains for upsizing and looping, deficiencies located near service provided to Dash Point State Park	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Old Town 346	1,638	732	Residential	1,500	Single Family Residental	Tacoma	1,000	2	69	Additional Analysis Required	Review 4" and 6" mains for upsizing	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Portland Avenue 346	3,599	551	Residential	1,500	Multi-Family (Low Density)	Tacoma	1,000	96	424	Additional Analysis Required	Review early 1900's 4" and 6" mains for upsizing	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Narrows 328	3,546	938	Residential	1,500	Multi-Family (Low Density)	Tacoma	1,000	1	640	Additional Analysis Required	Review zone boundary to determine if changing zones will improve fire flow, 1 node is identified	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required

						Max	kimum Day Dema	ind + Fire Flow As	sessment					
Pressure Zone and HGL Elevation (ft)	Maximum Available FF (gpm)	Minimum Available FF (gpm)	Basic Land Use Type	Tacoma Water Land Use Planning Level FF Target (gpm)	Juristiction Land Use Type	Juristiction	Juristiction Required FF (gpm)	Nodes Below 20 psi During FF	Total Nodes in Zone	Overall Assessment	Recommended Action/ Comment	2017 Assessment	2027 Assessment	2037 Assessment
High Cedars 316	3,468	1,678	Industrial	5,000	Rural Industrial Center	Pierce County	2,000	0	423	Adequate	Multiple land use types within zone, fire flow requirements are met for each type	Adequate	Adequate	Adequate
Chambers Bay 290	2,653	2,450	City	3,500	University Place	Pierce County	1,500	0	17	Adequate	None	Adequate	Adequate	Adequate
Low Service 251	62,649	378	Industrial	5,000	Heavy Industrial	Tacoma	2,000	8	3,361	Additional Analysis Required	Three locations identified with fire flow restrictions, review 4" and 6" CI mains for upsizing	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Dash Low Point 226	1,076	773	Residential	1,500	Single Family	Pierce	750	0	35	Adequate	None	Adequate	Adequate	Adequate
Hyada 226	3,407	864	Residential	1,500	Single Family	Pierce	750	0	152	Adequate	None	Adequate	Adequate	Adequate
Titlow 226	2,995	1,149	Residential	1,500	Single Family Residental	Tacoma	1,000	0	122	Adequate	None	Adequate	Adequate	Adequate
Day Island 202	1,464	534	City	3,500	University Place	Pierce County	1,500	35	53	Additional Analysis Required	Review 4" and 6" mains for upsizing	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Lakota Beach 186	1,023	517	Residential	1,500	Single-Family Residential	King	1,000	21	62	Additional Analysis Required	Review 6" AC mains for upsizing, this zone appears to be undersized to provide 1,000 gpm	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required
Sunset Beach 155	705	700	City	3,500	University Place	Pierce County	1,500	2	6	Additional Analysis Required	4" DI pipe in PRV station should be assessed for replacement, it appears to be reducing available fire flow	Additional Analysis Required	Additional Analysis Required	Additional Analysis Required



Chapter 6, Appendix I: Distribution System Analysis

## Chapter I4. Technical Memo (RH2, 9/20/2017), Hydraulic Model Updates



# RH2 TECHNICAL

Client:	Tacoma Water		
Project:	Hydraulic Model Update and	d Calibration	
Project File:	TAC 517.122.01.102	Project Manager:	Michele Campbell, PE
Composed by:	Ryan Withers, PE		
Reviewed by:	Geoff Dillard, PE		
Subject:	Stage 1 Updates		
Date:	September 20, 2017		



This technical memorandum summarizes the updates made in Tacoma Water's hydraulic model by RH2 Engineering, Inc., (RH2) as part of the Stage 1 updates requested by Tacoma Water. Stage 1 consists of the following six updates.

- 1. Add minor losses to the 25 booster pump stations (BPS) in the hydraulic model.
- 2. Review and update check valve and variable frequency drive (VFD) functionality.
- 3. Compute pipe roughness coefficients and compare to existing pipe roughness coefficients.
- 4. Initialize "no services" and "facility" junctions.
- 5. Initialize planning-level fire flow requirements.
- 6. Create peak hour demand (PHD) demand sets.

## BACKGROUND

In 2016, Tacoma Water created a new InfoWater hydraulic model from Tacoma Water's ESRI Geographical Information System (GIS) database. In 2016 and 2017, Tacoma Water updated the model to include demand allocations, simple controls at existing facilities, and additional water system properties such as pressure zone information, pipe roughness coefficients, land use information, and reservoir geometry. Tacoma Water is preparing to utilize the hydraulic model for system analyses in its 2017 *Water System Plan* and requested that RH2 perform the Stage 1 updates using InfoWater version 12.7, Update No. 2.

## **UPDATE 1 – BPS MINOR LOSSES**

Minor losses occur throughout water systems at valves, tees, bends, reducers, and other fittings, and can be significant within facilities where water velocities are typically higher than in the transmission or distribution system. Based on conversations between Tacoma Water and RH2 at the project scoping meeting on July 14, 2017, Tacoma Water estimated minor losses in each of its BPSs to be 10 feet in the hydraulic model, with 3 feet of minor losses assumed on the suction side of each pump, and 7 feet of minor losses assumed on the discharge side of each pump. RH2 initialized the minor losses at each BPS based on the design capacity of each BPS, which is shown in **Table 1**. The minor losses were entered into the Tacoma Water hydraulic model as resistance coefficients (K values) on the suction and discharge pipes at each BPS. The resistance coefficient added to the suction and discharge pipes at each BPS is shown in **Table 1**.

			Su	ction Piping	g	Dise	charge Pipi	ng
BPS Name	Capacity (MGD)	Capacity (gpm)	Model Pipe ID	Pipe Size (inches)	Resistance Coefficient (K Value)	Model Pipe ID	Pipe Size (inches)	Resistance Coefficient (K Value)
Alder Lane	0.7	486	M-0106940	4	1.3	M-0106941	4	2.9
Cumberland	0.5	347	M-0061645	6	12.4	M-0026032	6	29.0
Fennel Creek	3.4	2,361	M-0056905	12	4.3	M-0106532	12	10.0
Frederickson	0.2	139	M-0106660	6	77.8	M-0059547	6	181.5
Highland	1.0	694	M-0078868	8	9.8	M-0078896	8	22.9
Hood Street	15.0	10,417	M-0070401	48	56.6	M-0070323	24	8.3
Indian Hill No. 1	2.2	1,528	M-0068974	10	5.0	M-0068982	10	11.6
Indian Hill No. 2	1.9	1,319	M-0068302	12	13.8	M-0068310	12	32.2
Marine View Drive (1 - 4)	3.7	2,569	M-0099228	12	3.6	M-0056729	12	8.5
Marine View Drive (5 - 7)	7.0	4,861	M-0099199	12	1.0	M-0099005	16	7.5
McMillin No. 1	3.3	2,292	M-0105953	12	4.6	M-0062892	12	10.7
McMillin No. 2	4.2	2,917	M-0062683	24	45.2	M-0062705	24	105.4
Mildred Street	1.2	833	M-0099320	12	34.6	M-0105915	12	80.7
North End	8.5	5,903	M-0106336	24	11.0	M-0107063	24	25.7
Palmer	0.1	69	M-0105167	2	3.8	M-0105173	2	9.0
Prairie Ridge	1.8	1,250	M-0106458	18	77.8	M-0067828	18	181.5
			M-0056558	14	5.3			
South Tacoma	14.4	10,000	M-0056559	14	5.3	M-0106320	30	21.9
South Tacoma	14.4	10,000	M-0056560	14	5.3	IVI-0106320	30	21.9
			M-0056561	14	5.3			
South Summit	2.2	1,528	M-0056749	8	2.0	M-0056750	8	4.7
128th and 62nd (Summit and 62nd	0.6	417	M-0056773	6	8.6	M-0056774	6	20.2
198th Avenue	3.4	2,361	M-0106468	18	21.8	M-0106467	18	50.9
214th Avenue E	8.6	5,993	M-0056889	16	2.1	M-0056879	8	0.3
356th/357th Street	5.8	4,028	M-0106301	24	23.7	M-0106294	24	55.2
83rd and Cirque	0.5	347	M-0106930	6	12.4	M-0056543	8	91.8
80th Avenue	0.03	21	M-0101721	4	682.9	M-0101715	4	1593.4
21st and Pearl	3.3	2,257	M-0028955	12	4.7	M-0106922	12	11.0

**Table 1: BPS Resistance Coefficients** 

MGD = million gallons per day

## **UPDATE 2 – CHECK VALVE AND VFD FUNCTIONALITY**

#### CHECK VALVE FUNCTIONALITY

The Tacoma Water hydraulic model includes 112 check valves assigned to pipes throughout the distribution system; however, Tacoma Water indicated that the hydraulic model does not properly compute with the active check valves, and has closed the pipes that are associated with the check valves. Among the 112 check valves, 15 are located at facilities, and 97 are located along pressure zone boundaries. RH2 reviewed and updated the valves to enable the model to properly compute and the valves to function in the model in a manner that matches their function in the actual water system. All facility check valves were determined to be oriented in the proper direction and the pipes associated with these valves have been opened in the hydraulic model.

Among the 97 check valves located at pressure zone boundaries, 42 were found to be oriented in the incorrect direction. The direction of these check valves has been updated, and 96 of the 97 check valves located at zone boundaries have been activated in the hydraulic model. One check valve, assigned to Pipe ID No. M-0107007, appears to be located within the 538 Zone distribution system and is believed to have been erroneously identified as a check valve location. RH2 removed the check valve assignment from this pipe, and updated the pipe to operate as a pipe capable of two-way conveyance. It is recommended that Tacoma Water confirm the lack of a check valve at this location. If a check valve is present at this location, it is recommended that Tacoma Water confirm the flow direction of the valve, and update this pipe if necessary.

An additional check valve, assigned to Pipe ID No. M-0106929, is located at the convergence of three pressure zones near the 83<sup>rd</sup> Avenue and Cirque Drive BPS. RH2 opened the pipe associated with this check valve based on the assumption that the check valve allows 531 Zone water to be conveyed to the 556 Zone during an emergency event. It is recommended that Tacoma Water confirm the flow direction of this valve, and update this check valve if necessary.

**Table 2** presents a summary of each check valve in the hydraulic model, the pressure zone on each side of the valve, and the changes made to the valve direction (if any). The aforementioned pipes recommended for review by Tacoma Water are shown highlighted in **Table 2**.

		Pressu	re Zone			
Pipe No.	Zone or Facility?	То	From	Direction Correct?		
M-0106934	Facility	626	581	Yes		
M-0106407	Facility	669	581	Yes		
M-0106467	Facility	198th Ave B	PS Discharge	Yes		
M-0107052	Facility	Sunrise F	Res. Outlet	Yes		
M-0107053	Facility	Sunrise	Res. Inlet	Yes		
M-0065881	Facility	McMillin F	Res. Outlet	Yes		
M-0106958	Facility	Palmer Bl	PS Bypass	Yes		
M-0106957	Facility	Palmer BP	S Discharge	Yes		
M-0107066	Facility	North E	ind BPS	Yes		
M-0106327	Facility	Trans	538	Yes		
M-0106956	Facility	Trans	931	Yes		
M-0107051	Facility	Trans	McMillin Res.	Yes		
M-0107020	Facility	UP Reservoir	rs to 478 Zone	Yes		
M-0106831	Facility	Well PA1	Discharge	Yes		

Table 2: Check	Valve Summary
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Table 2: Check Valve Summary (continued)				
		Press	_	
Pipe No.	Zone or Facility?	То	From	Direction Correct?
M-0107054	Zone	328	226	Yes
M-0107055	Zone	328	226	Yes
M-0106974	Zone	346	251	Yes
M-0107017	Zone	346	251	No, RH2 changed
M-0107025	Zone	346	251	Yes
M-0106995	Zone	478	328	Yes
M-0107032	Zone	478	346	No, RH2 changed
M-0106994	Zone	478	446N	Yes
M-0107023	Zone	478	446N	Yes
M-0107027	Zone	478	446N	No, RH2 changed
M-0107004	Zone	520	478	Yes
M-0107037	Zone	520	478	No, RH2 changed
M-0107040	Zone	520	478	Yes
M-0107057	Zone	520	478	Yes
M-0107058	Zone	520	478	Yes
M-0000860	Zone	531	478	Yes
M-0106959	Zone	531	478	Yes
M-0106961	Zone	531	478	No, RH2 changed
M-0106964	Zone	531	478	Yes
M-0107015	Zone	531	478	Yes
M-0107022	Zone	531	478	No, RH2 changed
M-0107059	Zone	531	478	No, RH2 changed
M-0106338	Zone	538	478	Yes
M-0106924	Zone	538	478	No, RH2 changed
M-0106975	Zone	538	478	No, RH2 changed
M-0106976	Zone	538	478	No, RH2 changed
M-0106977	Zone	538	478	Yes
M-0106979	Zone	538	478	No, RH2 changed
M-0106980	Zone	538	478	No, RH2 changed
M-0106981	Zone	538	478	No, RH2 changed
M-0106982	Zone	538	478	No, RH2 changed
M-0106983	Zone	538	478	No, RH2 changed
M-0106984	Zone	538	478	Yes
M-0106985	Zone	538	478	No, RH2 changed
M-0106986	Zone	538	478	No, RH2 changed
M-0106988		538	478	No, RH2 changed
	Zone			· · · · · · · · · · · · · · · · · · ·
M-0106988	Zone	538	478 478	No, RH2 changed
M-0106989	Zone	538		Yes
M-0106990	Zone	538	478	Yes
M-0106991	Zone	538	478	Yes
M-0106992	Zone	538	478	Yes
M-0106993	Zone	538	478	No, RH2 changed
M-0107005	Zone	538	478	No, RH2 changed
M-0107006	Zone	538	478	Yes
M-0107008	Zone	538	478	Yes
M-0107010	Zone	538	478	No, RH2 changed
M-0107011	Zone	538	478	No, RH2 changed
M-0107012	Zone	538	478	Yes
M-0107014	Zone	538	478	Yes

Table 2: Check Valve Summary (continued)

	Pressure Zone			
Pipe No.	Zone or Facility?	То	From	Direction Correct?
M-0107018	Zone	538	478	Yes
M-0107019	Zone	538	478	No, RH2 changed
M-0107026	Zone	538	478	No, RH2 changed
M-0107007	Zone	538	538	Not Applicable
M-0106962	Zone	556	478	Yes
M-0106960	Zone	556	531	No, RH2 changed
M-0106963	Zone	556	531	Yes
M-0107016	Zone	556	531	Yes
M-0107021	Zone	556	531	No, RH2 changed
M-0106972	Zone	581	478	Yes
M-0106973	Zone	581	478	No, RH2 changed
M-0106996	Zone	581	478	Yes
M-0106997	Zone	581	478	Yes
M-0107024	Zone	581	478	Yes
M-0107028	Zone	581	478	No, RH2 changed
M-0107035	Zone	581	478	No, RH2 changed
M-0107036	Zone	581	478	Yes
M-0106999	Zone	588	581	No, RH2 changed
M-0107043	Zone	588	581	Yes
M-0069363	Zone	621	581	Yes
M-0106418	Zone	621	581	Yes
M-0107003	Zone	621	581	Yes
M-0106691	Zone	626	581	No, RH2 changed
M-0106952	Zone	626	581	Yes
M-0107039	Zone	626	581	No, RH2 changed
M-0106965	Zone	649	549	No, RH2 changed
M-0106966	Zone	649	549	No, RH2 changed
M-0106967	Zone	649	549	Yes
M-0106968	Zone	649	549	No, RH2 changed
M-0106969	Zone	649	549	No, RH2 changed
M-0106970	Zone	649	549	No, RH2 changed
M-0106971	Zone	649	549	No, RH2 changed
M-0107031	Zone	649	549	Yes
M-0107033	Zone	649	549	Yes
M-0107034	Zone	649	549	No, RH2 changed
M-0107000	Zone	669	581	Yes
M-0107001	Zone	669	581	Yes
M-0107002	Zone	669	581	Yes
M-0107029	Zone	669	581	Yes
M-0107030	Zone	669	581	Yes
M-0107038	Zone	669	581	Yes
M-0107041	Zone	669	581	Yes
M-0107042	Zone	669	581	Yes
M-0107050	Zone	669	581	Yes
M-0106455	Zone	706	581	Yes
M-0106615	Zone	706	621	Yes
M-0106998	Zone	950	860	No, RH2 changed
M-0107056	Zone	Trans	Trans	Yes
M-0106929	Zone/Facility	556	531	Yes

Table 2: Check Valve Summary (continued)

NOTE: Highlighted rows indicate check valves recommended for review by Tacoma Water.

#### **VFD FUNCTIONALITY**

The Tacoma Water hydraulic model includes 25 booster pump stations, 10 of which have at least 1 pump equipped with a VFD. Tacoma Water indicated that the hydraulic model does not properly compute with all BPSs and VFDs active. RH2 reviewed the pumps and updated them to enable the model to properly compute and the pumps to function in the model in a manner that matches their intended function in the actual water system.

Among the 25 BPSs, 4 were set as inactive by Tacoma Water and were not activated by RH2. None of these four BPSs had control settings indicating that the pumps were equipped with VFDs, and the operation of these pumps were not evaluated by RH2. These four BPSs are identified in **Table 3**.

Hood Street Marine View Drive (1 - 4) Marine View Drive (5 - 7)	No No No	Current Initial Status d Not Evaluated to Inactive Inactive Inactive		Pump Control	
South Tacoma BPSs Op		Inactive erly and Not Adjus			
Alder Lane	Yes	1 Pump Open	70	Open	
Cumberland	No	Pumps Open		Cumberland Res.	
Fennel Creek	Yes	1 Pump Open	99	Open	
Frederickson	Yes	1 Pump Open	55	Open	
Highland	Yes	1 Pump Open	73	Open	
Indian Hill No. 1	Yes	Pumps Open	45	Open	
Indian Hill No. 2	No	Closed		Open	
North End	No	Closed		Open	
Palmer	No	Closed		Open	
Prairie Ridge	Yes	1 Pump Open	95	Open	
South Summit	Yes	1 Pump Open	90	Open	
128th and 62nd (Summit & 62nd Ave)	No	Closed		Open	
198th Avenue	No	Pumps Open		Prairie Ridge Res.	
214th Avenue E	No	3 Pumps Open		Prairie Ridge Res.	
356th/357th Street	No	Closed		Open	
83rd and Cirque	Yes	Pump Open	55	Open	
BPSs Adjusted by RH2					
McMillin No. 1	No	Pumps Open		Sunrise Standpipe	
McMillin No. 2	No	Pumps Open		Sunrise Standpipe	
Mildred Street	No	Pump Open		Open	
80th Avenue	Yes	1 Pump Open	72	Open	
21st and Pearl	Yes	Pump Open	65	Open	

Table 3: VFD	Summary
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Among the 21 BPSs that were set as active in the model received by RH2, 16 were determined to be operating properly and were not adjusted by RH2. These 16 BPSs are identified in **Table 3**.

The remaining five BPSs were adjusted by RH2 for the model to properly compute and the pumps to function in the model in a manner that matches their function in the actual water system. These five BPSs are identified in **Table 3**, and a summary of the adjustments made by RH2 is as follows.

#### McMillin No. 1 and No. 2 BPSs

Both the McMillin No. 1 BPS and the McMillin No. 2 BPS had two pumps closed in the model received by RH2. RH2 changed the initial status of these pumps to "none" to allow them to operate during normal operating conditions. The proper initialization of the check valves in the vicinity of the McMillin BPS and Reservoir site prevents the reverse flow through the McMillin No. 1 BPS and the McMillin No. 2 BPS that was occurring in the model received by RH2.

#### **Mildred Street BPS**

The Mildred Street BPS was inactive and labeled as "Temp\_OOS" (temporarily out of service) in the model received by RH2. RH2 activated this BPS to operate during normal operating conditions.

#### 80th Avenue BPS

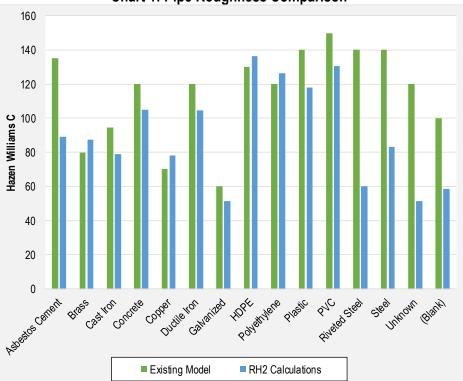
The 80<sup>th</sup> Avenue BPS provides supply to a closed pressure zone. Both pumps within this BPS were identified as being equipped with VFDs and had pump types identified as "constant power pumps," with one pump identified as having a 2 horsepower (HP) motor, and the other pump identified as having a 1 HP motor. By definition, "constant power pumps" cannot have variable speeds. RH2 updated the pump types for both pumps in this BPS to have "design point curve" pump types. The total dynamic head for each pump was assumed to be 45 feet based on the hydraulic grade differential between the 581 South Hill Zone on the suction side of the BPS and the 626 80<sup>th</sup> Avenue E Zone on the discharge side of the BPS. Based on the 1 HP and 2 HP motor sizes identified in the model received by RH2, and an assumed pump efficiency of 70 percent for each pump, the pump design flow for the two pumps was initialized as 60 gallons per minute (gpm) and 120 gpm, respectively. RH2 initialized the VFD control for both pumps as 70 pounds per square inch (psi) to match the hydraulic grade of the 626 80<sup>th</sup> Avenue E Zone, and the BPS now operates properly in the model.

#### 21st and Pearl BPS

The 21<sup>st</sup> and Pearl BPS pump was closed in the model received by RH2. RH2 changed the initial status of this pump to "none" to allow it to operate during normal operating conditions.

## **UPDATE 3 – PIPE ROUGHNESS COEFFICIENTS**

Hazen Williams roughness coefficients were entered into the model by Tacoma Water based on roughness calculations and research performed over 10 years ago, per Tacoma Water staff at a July 14, 2017, scoping meeting with RH2. Tacoma Water requested that RH2 compare the hydraulic model roughness coefficients with roughness coefficients based on recent research and measurements performed on pipe interiors by RH2. RH2's computations assume that the internal surface of the pipes becomes rougher as it ages; therefore, older pipes were assigned higher roughness coefficients than newer pipes. The computed RH2 pipe roughness coefficients were compared to the values entered by Tacoma Water based on the average pipe roughness coefficient for each pipe material. The pipe roughness comparison for each pipe material is shown in **Chart 1**. Among the 15 different pipe materials identified in the hydraulic model received by RH2 (including unknown and blank materials), the roughness coefficients calculated by RH2 are within 20 percent of the coefficients entered by Tacoma Water for 10 of the materials. The other five pipe materials, asbestos cement, riveted steel, steel, unknown, and blank, have roughness coefficients in the model that differ from RH2's calculations by an average of 46 percent. RH2 presented this information to Tacoma Water during an August 30, 2017, kickoff meeting. Tacoma Water requested that RH2 update the hydraulic model roughness coefficients for these five pipe materials to match the values calculated by RH2, and not update the coefficients for the other ten materials. RH2 revised the roughness coefficients for pipes identified with asbestos cement, riveted steel, steel, unknown, and blank materials in the hydraulic model, and will further evaluate pipe roughness coefficients during the Stage 2 (steady-state hydraulic model calibration) portion of the contract.





## **UPDATE 4 – "NO SERVICES" AND "FACILITY" JUNCTIONS**

Junctions throughout the hydraulic model were assigned pressure zone information by Tacoma Water based on the pressure zone the junction is physically within, or identified as being a junction on a transmission main ("Trans"). Junctions with assigned pressure zones that are located either at facilities or in locations without services can limit the fire flow availability calculated by the model during batch fire flow analyses, as these junctions are typically located in areas where the residual pressure is less than the minimum pressure constraints identified in the fire flow simulation options (typically 20 psi). Examples of junctions in these areas include junctions near a tank or reservoir, or on the suction side of a pump, RH2 updated the pressure zone assignment of more than 1,100 junctions as "facility" if the junction is located near a facility, or "no services" if the junctions are located elsewhere in the water system in a location without any water services. Junctions not located near a facility that were not assigned pressure zones by Tacoma Water were not evaluated or updated by RH2, as it is outside of RH2's scope to do so. Junctions that have pressures below 25 psi in the existing average day demand (ADD) scenario (QS NW 2017 PADD scenario) that were not updated by RH2 because there appear to be active water services near these locations, are shown in Table 4. Following initialization of the fire flow analyses to be performed by Tacoma Water in fall 2017, it is recommended that Tacoma Water review the junctions that limit the fire flow availability in each pressure zone and determine if any additional junctions need to be assigned to the "no services" or "facility" pressure zone.

Table 4: Low Pressure Junctions with Pressure Zone Assignments				
Junction ID No.	Junction Elevation (feet)	Hydraulic Grade (feet)	Pressure (psi)	Pressure Zone
J38468	392.6	436.9	19.2	446N
J39414	391.4	436.9	19.7	446N
J125988	429.1	474.9	19.8	478
J21468	429.1	474.9	19.8	478
J19768	429.1	474.9	19.8	478
J19770	429.1	474.9	19.9	478
J38660	429.2	477.1	20.7	478
J127778	429.0	477.0	20.8	478
J127780	429.0	477.0	20.8	478
J127782	429.0	477.0	20.8	478
J38670	429.0	477.0	20.8	478
J70092	429.0	477.0	20.8	478
J38672	428.9	477.0	20.9	478
J38674	428.9	477.0	20.9	478
J122512	428.8	477.1	20.9	478
J71312	428.8	477.1	20.9	478
J121732	427.9	477.0	21.3	478
J70150	427.9	477.0	21.3	478
J70158	427.9	477.0	21.3	478
J3652	424.4	476.0	22.4	478
J3654	424.4	476.0	22.4	478
J3678	424.4	476.0	22.4	478
J3616	424.1	476.0	22.5	478
J3650	424.1	476.0	22.5	478
J84720	422.9	474.9	22.5	478
J39416	381.7	436.8	23.9	446N
J47122	422.2	477.5	23.9	581 Fletcher Hts
J33478	385.3	442.6	24.8	478

Table 4: Low Pressure Junctions with Pressure Zone Assignments

## **UPDATE 5 – PLANNING-LEVEL FIRE FLOW REQUIREMENTS**

Planning-level fire flow requirements were identified by Tacoma Water for the different land use categories to provide a target level of service for planning for and sizing of future water facilities. The planning-level fire flow requirement for each land use category is shown in Table 5. RH2 created a "Basic\_LU" database set for junctions that have a basic land use assigned in the hydraulic model, and initialized the planning-level fire flow requirement for each junction in the fire flow table based on the land use categories shown in Table 5.

Land Use	Planning-level Fire Flow Requirement (gpm)	Duration (hours)
Agricultural	1,000	2
City	3,500	4
Commercial	5,000	5
Industrial	5,000	5
Mixed Use	3,500	4
Park	1,000	2
Residential	1,500	2
Residential High Density	3,500	4
Undesignated	3,500	4

Table 5: Planning-level	Fire Flow Requirements

## **UPDATE 6 – PHD DEMAND SETS**

The hydraulic model originally provided to RH2 included ADD and maximum day demand (MDD) demand sets for the 2017, 2027, and 2037 planning periods. Tacoma Water provided PHD/ADD peaking factors for various land uses and requested that RH2 create PHD demand sets for the 2017, 2027, and 2037 planning periods. The peaking factors provided by Tacoma Water are as follows.

- PHD/ADD = 3.11 for city, mixed use, residential, residential high density, and undesignated land uses.
- PHD/ADD = 1.16 for agricultural, commercial, industrial, and park land uses.

RH2 created the PHD demand sets and scenarios for the 2017, 2027, and 2037 planning periods based on these PHD/ADD peaking factors. Within the hydraulic model, the peaking factor applied to each demand allocation group is as follows.

- "Res," "Res-Multi," "Res-Irrig" = 3.11.
- "Commercial," "Com-Irrig," "Wholesale," "Simpson," "Partners" = 1.16.
- "Res-Fire," "Com-Fire" = 1.00 (no change between ADD and PHD).

The resulting system-wide PHDs for the 2017, 2027, and 2037 planning periods are shown in **Table 6**, as are the system-wide PHD/ADD peaking factors. The 2027 and 2037 ADD demand sets have lower demands than the 2017 ADD demand set; therefore, the 2027 and 2037 PHD

demands are less than the 2017 PHD. The PHD/ADD peaking factors differ for the 2017, 2027, and 2037 planning periods because the percentage of demands with a 3.11 PHD/ADD peaking factor is less in the future planning periods compared to the 2017 planning period.

Demand Type	System-wide Demand (gpm)	System-wide PHD/ADD Factor			
	2017 Planning Period				
2017 ADD	34,462	4.05			
2017 PHD	63,656	1.85			
2027 Planning Period					
2027 ADD	32,078	1.00			
2027 PHD	57,747	1.80			
2037 Planning Period					
2037 ADD	30,975	1 77			
2037 PHD	54,899	1.77			

#### Table 6: System-wide Demands

## **OTHER OBSERVATIONS**

While the six Stage 1 updates were being performed in the hydraulic model, RH2 identified numerous junctions and pipes with pressure zone assignments that differed from the actual pressure zone they actually convey water to. One example of this occurs near the intersection of 111<sup>th</sup> Avenue E and 128<sup>th</sup> Street E near the boundary between the 621 (Highland) Zone and the 581 (South Hill) Zone. Junctions on the west side of 111<sup>th</sup> Avenue E, directly west of the Highland BPS, are assigned to the 621 Zone, but are actually served by the 581 Zone, as shown in **Figure 1**.

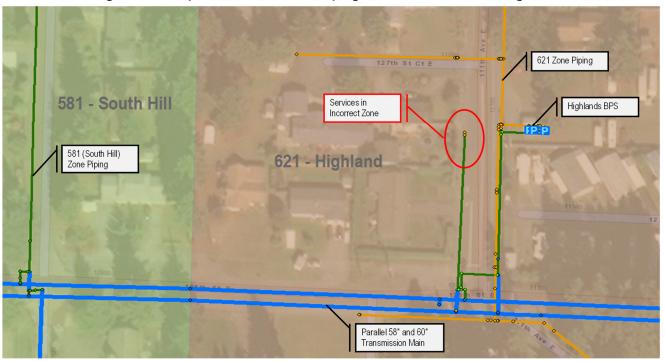
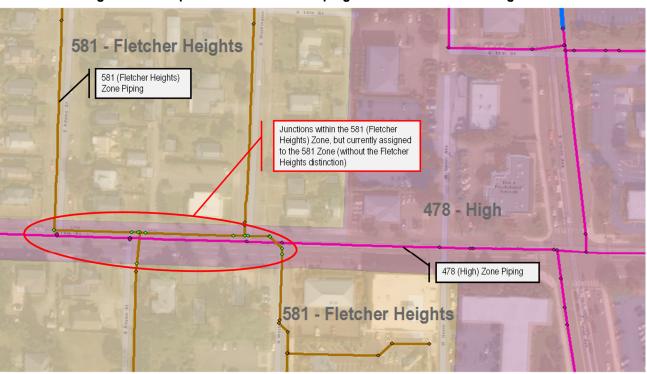


Figure 1: Example of Junctions and Piping with Incorrect Zone Assignments

Another example of pressure zone assignments recommended for additional review by Tacoma Water include pressure zones with the same hydraulic grade that are physically disconnected. The Tacoma Water distribution system provides service to multiple pressure zones with a 581-foot hydraulic grade. The pressure zone assignments for the junctions in these zones typically are indicated with a descriptor, such as "581 SE Tacoma" or "581 Fletcher Hts." However, there are instances wherein the 581 Zone assignments are incorrect. One example of this occurs along S 19<sup>th</sup> Street, west of the intersection with S Union Avenue near the boundary between the 581 (Fletcher Heights) Zone and the 478 (High) Zone, as shown in **Figure 2**. Junctions in the 581 (Fletcher Heights) Zone are properly assigned on either side of S 19<sup>th</sup> Street, but the junctions within S 19<sup>th</sup> Street currently have a "581" pressure zone assignment; therefore, they would not be considered as part of the minimum pressure check during fire flow analyses for the 581 (Fletcher Heights) Zone.





It is outside RH2's scope of work to resolve pressure zone discrepancies similar to those shown in **Figures 1** and **2** throughout the hydraulic model. Junctions with an incorrect pressure zone assignment may erroneously limit or over-report the fire flow availability throughout the pressure zone; therefore, it is recommended that Tacoma Water review the pressure zone assignments for junctions and pipes as part of the fire flow analyses to be set up and performed by Tacoma Water for inclusion in Tacoma Water's 2017 *Water System Plan*. Additionally, some junctions within the distribution system do not have pressure zone assignments. It is recommended that Tacoma Water include a pressure zone assignment for every junction in the model, including any additional junctions requiring a "no services" or "facility" assignment.

## CONCLUSION

RH2 is transmitting the hydraulic model with the six Stage 1 updates to Tacoma Water with this technical memorandum. RH2 recommends that Tacoma Water review the updates and contact RH2 with questions or comments regarding the updates prior to performing hydraulic analyses for inclusion in Tacoma Water's 2017 *Water System Plan*.



Chapter 6, Appendix I: Distribution System Analysis

## Chapter I5. Technical Memo (RH2, 2/14/2018), Hydraulic Model Steady State Model Calibration



Client:	Tacoma Water				
Project:	Hydraulic Model Update and Calibration				
Project File:	TAC 517.122.02.201	Project Manager:	Michele Campbell, PE		
Composed by:	Ryan Withers, PE				
Reviewed by:	Geoff Dillard, PE				
Subject:	Stage 2 Updates				
Date:	February 14, 2018				



This technical memorandum contains a description of the calibration of Tacoma Water's hydraulic model performed by RH2 Engineering, Inc. (RH2). The results of the calibration analyses and updates performed by RH2 in the hydraulic model are also contained in this technical memorandum.

## BACKGROUND

In 2016, Tacoma Water created a new InfoWater hydraulic model of its entire water system from Tacoma Water's ESRI Geographic Information System (GIS) database. In 2016 and 2017, Tacoma Water updated the model to include demand allocations, simple controls, and operational setpoints at existing facilities, as well as additional water system properties such as pressure zone information, pipe roughness coefficients, elevation data, land use information, and reservoir geometry. In September 2017, RH2 performed the Stage 1 hydraulic model updates, which consisted of the following six updates.

- 1. Add minor losses to the 25 booster pump stations (BPS) in the hydraulic model.
- 2. Review and update check valve and variable frequency drive (VFD) functionality.
- 3. Compute pipe roughness coefficients and compare to existing pipe roughness coefficients.
- 4. Initialize "no services" and "facility" junctions.
- 5. Initialize planning-level fire flow requirements.
- 6. Create peak hour demand (PHD) demand sets.

Following the Stage 1 updates, Tacoma Water requested that RH2 perform the Stage 2 updates using InfoWater version 12.7, which included identifying strategic locations in the water system to be field tested and using the results to calibrate the water model. The hydraulic model was calibrated for steady-state simulations to provide a greater degree of insight into the operation of the system.

## FIELD HYDRANT FLOW AND PRESSURE TESTING

Between October 2, 2017, and November 2, 2017, field flow tests were performed at 141 locations by RH2 and Tacoma Water staff. A subsequent single day of field flow tests were performed on January 18, 2018, at nine locations by RH2 and Tacoma Water staff. In total, 150 flow tests were performed at 147 unique locations, as three locations tested in January 2018 were the same locations tested in October and November 2017.

The location of each flow test is shown in **Figures 1** through **11**. Locations identified with a yellow symbol indicate the location of the static hydrant, and locations identified with an orange symbol indicate the location of the flowing hydrant. Locations identified with a green symbol were used for pressure transducers to monitor distribution system behavior before, during, and after the flow tests. The following is a summary of the flow testing procedures.

- The static pressure was measured and recorded at the static and flowing hydrants with all hydrants closed.
- One hose port on the flowing hydrant was opened. The flow from this port was measured and recorded, and the residual pressure at the static hydrant was recorded.
- If the measured pressure drop at the static hydrant was approximately 20 pounds per square inch (psi) or less, or the residual pressure exceeded approximately 40 psi, the second port of the flowing hydrant was opened. The flow from both ports was measured and recorded, and the residual pressure at the static hydrant was recorded.
- Both hose ports at the flowing hydrant were closed.
- The measurements obtained during each flow test were entered into a spreadsheet and are shown in the attached **Hydraulic Model Calibration Data** tables.

In total, 294 static pressure tests and 275 flow tests were performed at the 147 locations. At most locations, two static pressure tests were performed (one each at the residual and flowing hydrants). Two flow tests were also performed at most locations (one with a single hose port on the flowing hydrant open, and one with two hose ports on the flowing hydrant open). **Table 1** presents the total number of locations, static tests, and flow tests in each of Tacoma Water's operating areas. In general, each operating area includes multiple pressure zones.

Operating Area	No. of Unique Locations	No. of Static Tests	No. of Flow Tests	No. of Pressure Transducers
Bonney Lake	8	16	15	13
McMillin	10	20	17	11
South Hill	11	22	21	12
Canyon	8	16	16	10
SE Tacoma	14	28	28	20
478 High Zone	27	54	53	38
West and NW Tacoma	28	55	51	32
Port of Tacoma & Fife Heights	10	20	18	10
549 NE Tacoma Zone	11	22	21	16
Other NE Tacoma Zones	20	41	35	22
TOTALS	147	294	275	184

In addition to the data recorded at the static and flowing hydrants, data was recorded by pressure transducers strategically located throughout the distribution system during the flow tests, and Tacoma Water provided supervisory control and data acquisition (SCADA) data for facilities within the operating area that were being tested each day. Throughout the 25 days of hydrant flow testing, 184 pressure transducers were installed in the distribution system, each recording pressures at 2-second intervals. Based on an average deployment of approximately 5 hours per transducer, approximately 1.6 million pressure readings were recorded, analyzed and considered by RH2 during the calibration analyses. Similarly, Tacoma Water provided RH2 with SCADA data for 148 facilities encompassing the 25 days of hydrant flow testing, each with 1-minute recording intervals. The SCADA data primarily included reservoir water levels, pump flow rates, and suction and discharge pressures at facilities. In total, approximately 578,000 SCADA data points were provided to RH2 that were analyzed and considered during the calibration analyses. Although each individual pressure reading and SCADA data point were not evaluated during the steady-state calibration analysis, the data corresponding to each of the 294 static and 275 flow tests were evaluated and were essential to the overall success of the hydraulic model calibration, as described in subsequent sections of this technical memorandum.

## **CALIBRATION SUMMARY**

RH2 began the calibration analysis with the version of the hydraulic model created as part of the Stage 1 hydraulic model updates. RH2 prepared a technical memorandum documenting the Stage 1 updates, and provided it to Tacoma Water on September 20, 2017. Year 2017 average day demands, which were calculated and allocated by Tacoma Water, were utilized for calibration scenarios. After conducting hydraulic analyses with the model simulating each of the field tests, model results were compared with actual field results. The model was then calibrated by adjusting pipe roughness coefficients, elevations, and facility settings (pressure reducing valve (PRV) settings, BPS settings, etc.) to bring the model into closer calibration with the field results. In some situations, RH2 identified pipe network connectivity or facility setpoints differed in the field from those in the hydraulic model; these locations were revised in the model to reflect field conditions. The identification of these differences were generally the result of sensitivity analyses, which consist of iterative model adjustments to assist in troubleshooting the cause of a discrepancy between field measurements and model calculations. Examples of

sensitivity analyses performed for the Tacoma Water hydraulic model include temporarily opening or closing a PRV to identify if the PRV opened in the field but not in the model (or vice versa), temporarily adjusting the pressure setpoints of pumps or valves to determine the impact on system pressures, or temporarily closing pipes or adding check valves to pipes to confirm the presence of a zone valve or check valve at pressure zone boundaries.

The calibration goals selected for the static pressure readings were established to be consistent with the guidelines published by the Engineering Computer Applications Committee (ECAC) in 1999, which suggest that field pressure measurements be within plus or minus 2 psi for 90 percent of readings when compared to the pressures calculated by the model at the same location. The calibration goals selected for the dynamic pressure readings were established to be consistent with the guidelines published by Walski et al. (Walski) in the 2003 Advanced Water Distribution Modeling and Management book, which suggest that the hydraulic model be able to predict the hydraulic grade line (HGL) to within 5 to 10 feet at model calibration points during peak demands. The ECAC and Walski calibration guidelines represent two of the three guidelines described in the Washington State Department of Health (DOH) Water System Design Manual.

#### SYSTEM-WIDE SUMMARY – STATIC CONDITIONS

Of the static pressure measurements recorded in Tacoma Water's system, 91 percent are within plus or minus 2 psi of the static pressure calculated by the hydraulic model under the same conditions, as shown in **Table 2**. This level of accuracy meets the ECAC guideline of having at least 90 percent of field measurements being within plus or minus 2 psi of the model's calculations. The only area within the system with less than 90 percent of measurements meeting the ECAC guideline is the smaller zones in western and northwestern Tacoma, not including the 478 High Zone, wherein 76 percent of field measurements are within plus or minus 2 psi of the model's **Summary** in a subsequent section of this technical memorandum.

ECAC (+/- 2 psi: Static Tests)           Location         No. of Tests         No. within + or - 2 psi         % within + or - 2 psi						
Location	No. of Tests	No. within + or - 2 psi	% within + or - 2 psi			
Bonney Lake	16	15	94%			
McMillin	20	18	90%			
South Hill	22	21	95%			
Canyon	16	16	100%			
SE Tacoma	28	27	96%			
478 High	54	53	98%			
West and NW Tacoma	55	42	76%			
Port of Tacoma & Fife Heights	20	19	95%			
549 NE Tacoma	22	20	91%			
Other NE Tacoma Zones	41	37	90%			
TOTALS	294	268	91%			

#### SYSTEM-WIDE SUMMARY – DYNAMIC CONDITIONS

Ninety-eight percent of the dynamic tests performed in Tacoma Water's system resulted in the headloss measured in the field being within 5 to 10 feet of the hydraulic grade (4.33 psi) of the headloss calculated by the hydraulic model, as shown in **Table 3**. This level of accuracy results in all 275 tests meeting the Walski calibration guideline of the model predicting the hydraulic grade line to within 5 to 10 feet at model calibration points during peak demands, such as fire flows. The dynamic test results are described in more detail in the **Operating Area Summary** in a subsequent section of this technical memorandum.

Location	No. of Tests	No. within 5 feet	No. within 10 feet	% within 10 feet
Bonney Lake	15	11	15	100%
McMillin	17	9	17	100%
South Hill	21	19	21	100%
Canyon	16	8	16	100%
SE Tacoma	28	21	28	100%
478 High	53	39	53	100%
West and NW Tacoma	51	28	51	100%
Port of Tacoma & Fife Heights	18	14	18	100%
549 NE Tacoma	21	19	21	100%
Other NE Tacoma Zones	35	24	35	100%
TOTALS	275	192	275	100%
		70%	100%	

#### **OPERATING AREA SUMMARY**

#### Bonney Lake Area (Figure 1)

- Pressure Zones
  - o 705 Fennel Creek
  - o 810 Prairie Ridge
  - o 860 Bonney Lake
  - o 950 Bonney Lake
  - o 1010 Bonney Lake
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 15 of 16 (94 percent) static pressure measurements within plus or minus 2 psi.
  - Dynamic Pressure Measurements
    - Walski: 15 of 15 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. A portion of the 20-inch-diameter main that connects the 810 Prairie Ridge Zone to the Prairie Ridge Reservoir was found to be disconnected between

Junctions J91116 and J81816. Pipe M-0109952 was created to connect these two junctions, with the same properties as the pipes on either side of these junctions.

- b. The pressure sustaining valve in the 214<sup>th</sup> BPS was allowing approximately 2,600 gallons per minute (gpm) into the 48-inch-diameter transmission main while also not sustaining any pressure or otherwise providing hydraulic benefit during static or dynamic analyses. This valve was closed for the calibration analyses and in the model accompanying this technical memorandum. It is recommended that this valve be inspected in the field for proper operation before it is opened in the model for future analyses.
- c. A minor loss coefficient of 6.0 was initialized for the piping in the 198<sup>th</sup> Avenue E and Cascadia Boulevard E PRV station.
- d. The Tahaleh 860 PRV station setpoints were changed from 69 and 72 psi, to 73 and 76 psi.

#### McMillin Area (Figure 2)

- Pressure Zones
  - 316 High Cedars
  - o 519 Sunrise Terrace
  - o 621 Highland
  - o 706 McMillin
- Calibration Progress
  - o Static Pressure Measurements
    - ECAC: 18 of 20 (90 percent) static pressure measurements within plus or minus 2 psi.
  - Dynamic Pressure Measurements
    - Walski: 17 of 17 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. The 316 High Cedars Zone PRV station elevations increased by 9.2 feet, from 115.83 feet to 125.00 feet.
  - b. The 316 High Cedars Zone PRV station (142<sup>nd</sup> Avenue Court E and 128<sup>th</sup> Street E) has four PRVs that have setpoints that are hydraulically similar, and cause the model to not converge during dynamic analyses. Sensitivity analyses were performed to close varying combinations of PRVs to allow the model to converge. For the purposes of the calibration analyses, the PRV set at 82 psi needs to be closed, and either one of the PRVs set at 79 psi or 84 psi needs to be closed. The PRVs set at 79 psi and 82 psi are closed in the model accompanying this technical memorandum.
  - c. For Flow Test No. 9, pressure transducer data indicates that the PRV set at 86 psi was flowing during static conditions, and the PRV set at 79 psi was also flowing during both low and high flow dynamic conditions.
  - d. For Flow Test No. 10, pressure transducer data indicates that the PRV set at 86 psi was flowing during static conditions, and the PRV set at 84 psi was also flowing during both low and high flow dynamic conditions.

- e. All 316 High Cedars Zone ductile iron pipe Hazen Williams coefficients were increased to 125.
- f. All 706 McMillin Zone ductile iron pipe Hazen Williams coefficients were increased to 130.
- g. For Flow Test No. 17, the static pressures measured in the field differ by approximately 6 psi compared to the static pressures calculated by the hydraulic model. A review of elevations in Google Earth indicates that elevations in the model differ by approximately 10 feet in 111<sup>th</sup> Street Court E and in Shawnee Road E north of the intersection with 111<sup>th</sup> Street Court E. The elevations of the following junctions were updated in the model to match the Google Earth elevations.
  - i. J107254
  - ii. J107258
  - iii. J107260
  - iv. J107262
  - v. J107264
  - vi. J107280
  - vii. J107294
- h. The setpoint of the large (4-inch diameter) valve in the 136<sup>th</sup> Avenue and Military Road PRV was changed to 12.5 psi.
- i. The VFD control setpoint for the Highland BPS was changed from 73 psi to 76 psi.

#### South Hill Area (Figure 3)

- Pressure Zones
  - o 426 Woodland
  - o 581 Woodland
  - o 581 South Hill
  - $\circ$  626 80<sup>th</sup> Avenue E
  - o 626 Alder Lane
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 21 of 22 (95 percent) static pressure measurements within plus or minus 2 psi.
  - Dynamic Pressure Measurements
    - Walski: 21 of 21 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. All 581 South Hill Zone asbestos cement pipe Hazen Williams coefficients were increased by 10.
  - b. Elevations in the 626 80<sup>th</sup> Avenue E Zone were decreased by 3 feet.
  - c. The VFD control setpoint for the 80<sup>th</sup> Avenue BPS was changed from 70 psi to 68 psi.

- d. The VFD control setpoint for the Alder Lane BPS was changed from 70 psi to 75 psi.
- e. All 626 Alder Lane Zone asbestos cement pipe Hazen Williams coefficients were increased to 120.
- f. All 581 and 426 Woodland Zone asbestos cement pipe Hazen Williams coefficients were increased to 127.
- g. All 581 and 426 Woodland Zone ductile iron pipe Hazen Williams coefficients were increased to 135, except the ductile iron pipe within 72<sup>nd</sup> Avenue, which was increased to 140.
- h. The Woodland Avenue and 84<sup>th</sup> Street PRV station setpoints were 57 psi and 55 psi for the small and large valves, respectively. The small valve setpoint was changed to 59 psi, and the large valve setpoint was unchanged.

#### Canyon Road Area (Figure 4)

- Pressure Zones
  - o 581 Canyon
  - 588 Frederickson
  - o 669 S Summit High
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 16 of 16 (100 percent) static pressure measurements within plus or minus 2 psi.
  - Dynamic Pressure Measurements
    - Walski: 16 of 16 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. The Canyon BPS 30 horsepower (HP) VFD control maximum speed setting was changed from 1.2 to 1.0, and the additional parallel pumps were changed from 1 to 0 because only one large pump turned on during field flow testing. The Canyon BPS 5 HP pump was closed.
  - b. The Canyon BPS 30 HP VFD pressure setting was changed from 90 psi to 92 psi.
  - c. The Canyon BPS minor losses were not initialized previously in the Stage 1 portion of this project. Based on the field pressure measurements, a minor loss coefficient of 8.50 was initialized on the discharge side of the BPS (Pipe M-0056747), and a minor loss coefficient of 2.00 was initialized on the suction side of the BPS (Pipe M-0056749).
  - d. All 669 South Summit High Zone asbestos cement pipe Hazen Williams coefficients were increased to 115.
  - e. A fake reservoir (Reservoir "FAKE\_RES") located at the intersection of 66<sup>th</sup> Avenue and 156<sup>th</sup> Street Court was deactivated.
  - f. Test No. 34: The model was initially calculating approximately 20 psi of additional headloss during the high flow test compared to the headloss measured during the field testing. During field testing, the hydraulic grade at the static hydrant during the high flow test was approximately equal to the hydraulic grade

of the 581 South Hill Zone. Sensitivity analyses were performed to determine possible check valve and/or connection locations between the 669 South Summit High Zone and the 581 Canyon Zone. The analyses indicate that reverse flow was likely occurring through the 148<sup>th</sup> Street E and Woodland PRV station during Test No. 34. A check valve was added to the model at this PRV station to simulate the ability of this PRV station to have reverse flows during these conditions. Pipe M-0056430A was added to the model at this location to simulate the check valve.

## Southeast Tacoma Area (Figure 5)

- Pressure Zones
  - o 520 SE Tacoma
  - o 581 SE Tacoma
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 27 of 28 (96 percent) static pressure measurements within plus or minus 2 psi.
  - Dynamic Pressure Measurements
    - Walski: 28 of 28 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. The Well SE11 throttle control valve was changed to a flow control valve.
  - b. All 581 SE Tacoma Zone asbestos cement pipe Hazen Williams coefficients were increased by 15.
  - c. Model elevations in the vicinity of Flow Test No. 45 and data logger SET6 were increased by 4 feet to better correlate with field-measured pressures.
  - d. The Hazen Williams roughness coefficient of the 8-inch-diameter piping in the 108<sup>th</sup> Street Court E and 109<sup>th</sup> Street Court E loop located west of Golden Given Road E (at the Test No. 43 location) was decreased from 120 to 100.
  - e. Hazen Williams roughness coefficients for piping in the vicinity of Flow Test No. 46 were increased by 20.

## 478 High Zone (Figures 6, 7, 8, and 11)

- Pressure Zones
  - 478 High
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 53 of 54 (98 percent) static pressure measurements within plus or minus 2 psi.
  - o Dynamic Pressure Measurements
    - Walski: 53 of 53 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.

- Following the October and November 2017 flow testing, the field-measured residual pressures of the low and high flow tests performed at Flow Test Nos. 59, 60, and 61 were not within 10 feet of the residual pressures calculated by the hydraulic model. These three flow tests are located in the southeastern portion of the 478 High Zone, approximately bound by Interstate 5 to the north and west, by S 48<sup>th</sup> Street to the south, and by the east side of the 478 High Zone boundary to the east. Sensitivity analyses were performed to identify the discrepancy between the field measurements and the model calculations, but a single discrepancy source was not able to be identified that impacts this area enough to allow these three tests to calibrate within the Walski guideline. Tacoma Water water quality staff indicated that this area has historically experienced flow from unexpected directions when assessing chlorine residuals. Tacoma Water also indicated that there may be undocumented valves, some of which may be check valves, in this area of the system. Tacoma Water investigated the possibility of undocumented and closed valves in this area during the week of December 11, 2017, but no closed valves were found. Nine additional flow tests were performed in this area on January 18, 2018, including retesting Flow Test Nos. 59, 60, and 61 to confirm the previouslyrecorded data at these locations, and for comparison with the data recorded at the other six flow test locations. The additional data provided critical information that was used to identify the discrepancy between the field measurements and model calculations in this area, allowing all 478 High Zone tests to calibrate within the Walski guideline. These nine flow test locations are shown separately from the other 478 High Zone flow test locations in Figure 11.
- Model Updates for Steady State Calibration
  - a. A large percentage of the 478 High Zone distribution system consists of cast iron pipe that is over 60 years old. There are many short sections of ductile iron pipe in the hydraulic model at locations where hydrants, tees, and other fittings have been connected to the old cast iron main. A minor loss (K value) coefficient of 5.0 was added to all ductile iron pipes in west and northwest Tacoma (including the 478 High Zone) that have a length of 5 feet or less to represent the headlosses associated with the couplings and transitions to different pipe materials.
    - Ductile iron pipes with a length of 5 feet or less were not applied a minor loss (K value) coefficient of 5 in four locations within the 478 High Zone. These locations are as follows, wherein the minor loss (K value) coefficient was initialized as 0.
      - 1. Within 37<sup>th</sup> Street W, between Tahoma Place W and Grandview Drive W, in the vicinity of Flow Test No. 72.
      - 2. Within 53<sup>rd</sup> Street W, between Grandview Drive W and 95<sup>th</sup> Avenue Court W, in the vicinity of Flow Test No. 73.
      - 3. At the intersection of S Madison Street and S 69<sup>th</sup> Street, in the vicinity of Flow Test No. 75.

- 4. At the intersection 54<sup>th</sup> Avenue Court W and Cirque Drive, in the vicinity of Flow Test No. 76.
- ii. Cast iron pipe in the vicinity of Flow Test No. 65 was first initialized with Hazen Williams roughness coefficients of 70 and 75, depending on the age of the piping. All cast iron piping in the vicinity of Flow Test No. 65 was set to 70 for calibration analyses.
- b. The results of the January 2018 field testing, which including retests at Flow Test Nos. 59 through 61, and Flow Test Nos. 147 through 152, indicate that cast iron pipe in the southeastern portion of the 478 High Zone installed prior to 1945 has a significantly different conveyance capacity than cast iron pipe in the same area that was installed in or more recently than 1945. The Hazen Williams roughness coefficients of cast iron pipe were adjusted in the hydraulic model to reflect the field-measured data in two distinct areas of the southeastern portion of the 478 High Zone, which are shown in Figure 11 as Area A and Area B. Area A is approximately bound by Interstate 5 to the north and west, S 48<sup>th</sup> Street to the south, and S Thompson Avenue to the east. Area B is approximately bound by Interstate 5 to the north, State Route 7 and the Tacoma Rail right-of-way to the west, E 49<sup>th</sup> Street to the south, and by the east side of the 478 High Zone boundary to the east. The Hazen Williams coefficient adjustments differed in Areas A and B, and are as follows.
  - i. The Hazen Williams coefficient of all cast iron pipe installed prior to 1945 located within Area A were initialized to 45.
  - ii. The Hazen Williams coefficient of all cast iron pipe installed prior to 1945 located within Area B were initialized to 53.
  - iii. The Hazen Williams coefficient of all cast iron pipe installed in and after 1945 in Areas A and B were initialized to 120.

The relatively low Hazen Williams coefficients of 45 and 53 for cast iron pipe installed prior to 1945 is supported by data presented in Table 2.3 of Haestad Methods' *Advanced Water Distribution Modeling and Management*, which presents Hazen Williams coefficients for various pipe materials based on pipe age and the loss of carrying capacity in the pipe depending on the corrosiveness of the water being carried. Table 2.3 indicates that 6- and 12-inch-diameter cast iron pipe between 60 and 100 years old that has had "appreciable" or "severe" attack on the pipe would have Hazen Williams roughness coefficients ranging between 30 and 66.

A preliminary hydraulic model was provided to Tacoma Water in December 2017 that was used for Tacoma Water's Water System Plan hydraulic analyses. The December 2017 version of the hydraulic model included preliminary calibration of the 478 High Zone, but the Hazen Williams coefficients of the piping within Areas A and B in **Figure 11** had not yet been updated to reflect the values identified in (i), (ii), and (iii) of this section. These Hazen Williams coefficient adjustments were made within the model accompanying this technical memorandum.

# West and Northwest Tacoma Area (Pressure Zones Adjacent to the 478 High Zone) (Figures 6, 7, and 8)

- Pressure Zones
  - Figure 6
    - 346 Portland Avenue
    - 478 High
    - Figure 7
      - 155 Sunset Beach
      - 202 Day Island
      - 226 Titlow
      - 290 Chambers Bay
      - 328 Narrows
      - 351 Grandview
      - 478 High
      - 531 University Place
      - 556 Park Royal
    - Figure 8
      - 251 Low
      - 346 Old Town
      - 350 Salmon Beach
      - 446 Middle
      - 446 North End
      - 478 High
      - 538 Fletcher Heights
      - 538 Westgate
      - 581 Fletcher Heights
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 42 of 55 (76 percent) static pressure measurements within plus or minus 2 psi.
    - ECAC: 50 of 55 (91 percent) static pressure measurements within plus or minus 3 psi.
    - ECAC: 55 of 55 (100 percent) static pressure measurements within plus or minus 4 psi.
      - Many of the pressure zones within this operating area are served by PRV stations and include steep slopes adjacent to Puget Sound. Elevations in areas with steep slopes are more susceptible to inaccuracies because significant elevation changes can occur with slight changes to the X and Y coordinates of a junction or valve. Therefore, it is recommended that a more relaxed guideline of plus or minus 3 psi be considered for the static pressure measurements in these zones. It is also recommended that if water system infrastructure is being designed within these zones and the elevation or pressure is critical to the design, that a pre-design survey be performed to identify more accurate elevations in these

zones. As survey-level data is obtained in these areas for other purposes, it is recommended that the hydraulic model be updated accordingly.

- Dynamic Pressure Measurements
  - Walski: 51 of 51 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. The Mildred Street BPS was closed for all calibration analyses, consistent with the SCADA data provided for the field testing days in the 538 Westgate and Fletcher Heights Zone. The Mildred Street BPS was activated (initial status set to "none") in the model accompanying this technical memorandum.
  - b. Flow Test Nos. 78 through 90: The 531 University Place Zone was functioning approximately as a 581 Zone during field testing. Sensitivity analyses indicate that the North End Reservoir was likely not filling or filling at a relatively limited rate in order for the hydraulic grade line of the 531 University Place Zone to increase to approximately 581 feet. For the purposes of the calibration analyses, it was assumed that the North End Reservoir's inlet pipe was closed for all field tests performed on and before October 20, 2017, and open for all field tests performed on and after October 23, 2017 (no tests were performed on October 21<sup>st</sup> or 22<sup>nd</sup>). The North End Reservoirs' inlet pipe was opened in the model accompanying this technical memorandum.
    - i. Flow Test Nos. 78 and 79: The 83<sup>rd</sup> Avenue and Cirque Drive BPS was not operational during field testing due to the elevated hydraulic grade line in the 531 University Place Zone. Supply to the 556 Park Royal Zone was provided via a check valve at 83<sup>rd</sup> Avenue and the Cirque Drive BPS during field testing. The inlet pipe to the North End Reservoir was closed for the calibration analyses to allow the 531 University Place Zone to operate with a hydraulic grade of approximately 581 feet, consistent with the pressures measured during the field testing.
  - c. Flow Test No. 83: The Chambers Bay PRV station setpoint was changed to represent a hydraulic grade in the zone of approximately 330 feet, compared to the previous setting of 291 feet. The PRV station setpoints were changed to 46 psi and 39 psi for the small and large valves, respectively.
  - d. Flow Test No. 84: The minor loss coefficient in the piping at the 48<sup>th</sup> Street and Grandview Drive PRV station were increased from 5 to 10 (Pipe M-0105778), and the large valve setpoint was decreased 1 psi to 47 psi. The minor loss coefficient of Pipe M-0032836 at the 4300 Soundview PRV station was increased from 5 to 15.
  - e. Flow Test No. 85: The Day Island Road PRV station setpoint changed to represent a hydraulic grade in the zone of approximately 202 feet, compared to the previous setting of 199 feet. The PRV station setpoints were increased by 1 psi for all valves. The minor loss coefficients were set to 0 for all Day Island Zone distribution pipes. All ductile iron pipe Hazen Williams roughness

coefficients were initialized to 120, and all cast iron pipe Hazen Williams roughness coefficients were initialized to 80.

- f. Flow Test No. 86: The elevation of Junction J44108, the static hydrant during Flow Test No. 86, is shown as 58 feet in Google Earth, which is approximately 9 feet lower than the elevation previously initialized in the hydraulic model. The elevation of this junction was set to 58 feet in the hydraulic model to match the Google Earth elevation. Other elevations in the vicinity of Junction J44108 were reviewed in Google Earth and compared to the hydraulic model elevations and were found to be within approximately 1 foot in all instances. No other junction elevations were updated.
  - i. Minor loss coefficients of piping at the intersections of S 19<sup>th</sup> Street and 88<sup>th</sup> Avenue W, and Walters Road and 88<sup>th</sup> Avenue W, were set to 0.
- g. Flow Test Nos. 91 through 95: With the North End Reservoir filling during calibration analyses, the hydraulic grade in the 538 Fletcher and Westgate Zones decreases by approximately 10 psi across zone from southeast to northwest. This hydraulic grade decrease was not occurring during field testing based on the static pressure measurements and transducer data. Although no SCADA data other than reservoir level at the NE Reservoir site is available, the field-measured static and dynamic pressures indicate that the North End BPS was operational during field testing. North End BPS pump 1 was turned on in the hydraulic model for Flow Test Nos. 91 through 95, and was returned to its initialized state (closed) in the model accompanying this technical memorandum.
  - i. If this assumption is incorrect and the North End BPS was not operational during field testing, the hydraulic grade in the transmission line supplying the 538 Fletcher and Westgate Zones was at least 5 feet higher during the field testing than is calculated by the hydraulic model. A review of the SCADA data, if available, from the source of the transmission line could determine if the upstream hydraulic grade of the transmission line is greater than is initialized in the hydraulic model, or if a lower flow rate is being conveyed through the transmission main than is calculated by the model.
- h. Flow Test No. 93: Two disconnected pipe sections were identified.
  - A 6-inch-diameter pipe in N 18<sup>th</sup> Street, near the intersection with N Ferdinand Street, was disconnected between Junctions J69780 and J36428. Pipe M-0014817a was added between these junctions with the same properties as Pipe M-0014817 immediately to the east.
  - ii. An 8-inch-diameter pipe in 6<sup>th</sup> Avenue, near the intersection with S Adams Street was disconnected between Junctions J83778 and J47070. Pipe M-0019042a was added between these junctions with the same properties as Pipe M-0019042 immediately to the east.
- i. Flow Test No. 95: The check valve direction of Pipe M-0106987 was reversed.
- j. Flow Test Nos. 101 through 103: The Alaska Street Reservoir water level data was not provided. Instead, transducer data from location D3 was used to initialize the Alaska Street Reservoir water level to 30 feet for the calibration analyses.

- i. The Hazen Williams roughness coefficient of the cast iron pipe water main extending from the Alaska Street Reservoir to Yakima Avenue was reduced by 10, from 85 to 75.
- k. Flow Test Nos. 145 and 146: The Portland Avenue Zone is served by three active PRV stations in the hydraulic model. Besides the 2-inch-diameter lead PRV for the zone, located in the Portland Avenue and 38th Street E PRV station and set to a hydraulic grade of 346 feet, the other pressure reducing valves are set at the same or very similar hydraulic grades (341 feet, 338 feet, and 341 feet). With these setpoints, the hydraulic model calculates very minimal pressure drops during Flow Test Nos. 145 and 146, which is not consistent with the pressure measured in the field at both test sites and locations PA1 and PA2 that are immediately downstream of the PRVs. Sensitivity analyses were performed to attempt to determine if some of the valves either did not open during field testing or if their setpoints are significantly lower than were initialized in the model. The results indicate that the static pressures of the zone match very well between the field and the model, indicating that the 2-inch-diameter valve setpoint is accurate. The field results also indicate that significant (approximately 30 psi) headloss occurs between the Portland Avenue PRV station and location PA1, whereas only 6 to 10 psi of pressure drop was measured between the two easterly PRV stations (E 38<sup>th</sup> Street and T Street PRV station, and E 38<sup>th</sup> Street and Roosevelt PRV station) and location PA2. Sensitivity analyses were also performed with all PRVs functioning as initialized in the hydraulic model, but with closed valves at various locations on Portland Avenue, T Street, Roosevelt Avenue, and Fairbanks Street.
  - i. Calibration within the Walski guideline was achieved with the large (4-inch diameter) valve in the Portland Avenue and 38<sup>th</sup> Street E PRV station closed, as well as the 8-inch-diameter valve in the E 38th Street and Roosevelt Avenue PRV station closed. The 6-inch diameter E 38<sup>th</sup> Street and T Street PRV station valve was active and set at 57 psi; and the setpoint of the 2-inch diameter valve in the Portland Avenue PRV station was unchanged. These revised PRV setpoints were included in the model accompanying this technical memorandum. However, even though the resulting static and dynamic pressures at Flow Test Nos. 145 and 146 correlate well between the field and the model, it is unlikely that these initial modelling settings accurately reflect the actual field settings or system configuration. It is recommended that the pressure setpoint of these three PRV stations be confirmed. If the PRV setpoints are correct, a review of the pipe diameters and connectivity in this pressure zone, and an investigation into a possibly closed mainline valve is recommended to take place. Based on the current modeling settings, the static pressures in the zone are consistent between the field and model, and the dynamic pressures are consistent along Portland Avenue and north of Fairbanks Street. Based on the transducer located at PA2 during field testing, the model is calculating dynamic pressures approximately 10 psi lower than the field measurements along T Street and Roosevelt Avenue (and south of Fairbanks Street).

 ii. In addition to the three active PRV stations, the hydraulic model includes three additional 346 Portland Avenue PRV stations that are inactive and without setpoints in the hydraulic model. These PRV stations include the E 34<sup>th</sup> Street and Fairbanks Street PRV, the E 34<sup>th</sup> Street and N Street PRV, and the E Harrison Street and N Street PRV (which is physically disconnected from the 346 Portland Avenue Zone). The inactive status of these PRV stations was maintained as part of the calibration analyses.

## Port of Tacoma and Fife Heights Area (Figure 9)

- Pressure Zones
  - o 251 Low
  - 411 Fife Heights Low
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 19 of 20 (95 percent) static pressure measurements within plus or minus 2 psi.
  - o Dynamic Pressure Measurements
    - Walski: 18 of 18 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. Piping at the intersection of E 11<sup>th</sup> Street and Alexander Avenue was not connected in the hydraulic model. Pipe M-0032248B and Junction J128282 were added to connect the piping at this intersection. Pipe M-0032248 was also split as part of the reconfiguration of this intersection in the hydraulic model. The new pipe, M-0032248B, was assumed to have the same properties as the adjacent pipe in Alexander Avenue. Without this reconfigured intersection, Flow Test No. 110 does not calibrate within the Walski guideline, and the accuracy of Flow Test No. 111 is reduced. It is recommended that Tacoma Water review the piping configuration at the intersection of E 11<sup>th</sup> Street and Alexander Avenue to confirm that these changes accurately represent the existing system configuration.
  - b. Flow Test Nos. 112 and 113: The setpoint of the small and large valves in the Fife Heights PRV station (66<sup>th</sup> Avenue E and 4<sup>th</sup> Street E) were 54 and 52 psi, respectively. The large valve setpoint was changed to 50 psi as part of the calibration analyses.

## 549 NE Tacoma Zone (Figure 10)

- Pressure Zones
  - o 549 NE Tacoma
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 20 of 22 (91 percent) static pressure measurements within plus or minus 2 psi.
  - o Dynamic Pressure Measurements
    - Walski: 21 of 21 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. The Indian Hills 0.5 million-gallon (MG) Reservoir and associated piping and junctions were activated in the hydraulic model for calibration analyses, and were returned to its initialized state (inactive) in the model accompanying this technical memorandum.
  - b. The pressure reducing valve setpoints at the 356<sup>th</sup> Street BPS were reduced to 77 psi to match SCADA data for calibration analyses to reflect conditions during field testing, and was returned to its initialized setting (83 psi) in the model accompanying this technical memorandum. The control settings for these PRVs were changed to open when the Indian Hills 3.5 MG Reservoir water level reaches 16.0 feet, and to close when the Indian Hills 3.5 MG Reservoir water level reaches 19.2 feet to match SCADA data. The updated control settings were included in the model accompanying this technical memorandum.
  - c. A 6-inch-diameter pipe was disconnected between Junctions J13404 and J70738. Pipe M-0005591A was added between these junctions with the same properties as Pipe M-0005591, which is immediately adjacent to these junctions.
  - d. A 6-inch-diameter pipe was disconnected between Junctions J72908 and J72909. Pipe M-0031333A was added between these junctions with the same properties as Pipe M-0031333, which is immediately adjacent to these junctions.

# NE Tacoma Area (Pressure Zones Adjacent to the 549 NE Tacoma Zone) (Figure 10)

- Pressure Zones
  - o 186 Lakota Beach
  - o 226 Dash Point Low
  - o 226 Hayada
  - o 346 Browns and Dash Point
  - o 346 NE Tacoma
  - o 370 Overlook
  - o 411 Dash Point High
  - o 411 Twin Lakes
  - 426 Harbor View
  - 486 Beverly Heights

- o 549 NE Tacoma
- o 649 Indian Hill
- Calibration Progress
  - Static Pressure Measurements
    - ECAC: 37 of 41 (90 percent) static pressure measurements within plus or minus 2 psi.
  - o Dynamic Pressure Measurements
    - Walski: 35 of 35 (100 percent) of residual pressure measurements predict the hydraulic grade line differential between static and residual conditions to within 10 feet.
- Model Updates for Steady State Calibration
  - a. Flow Test No. 126: The 6-inch-diameter large valve in the NE 44<sup>th</sup> Avenue and 33<sup>rd</sup> Street NE PRV station was set at 33 psi, only 2 psi lower than the small (2-inch diameter) valve at the same station. Pressure transducer data indicates these setpoints are closer to 5 psi apart; therefore, the large valve was set to 30 psi.
  - b. Flow Test No. 136: Hazen Williams roughness coefficients of all pipes in the 226 Dash Point Low Zone were reduced by 20, and the minor loss coefficients in the discharge piping of each PRV station serving this zone were set to 20. Pressure transducer data indicates that one of the PRV stations did not open during the flow test. Sensitivity analyses indicate that the PRV station that did not open was the Markam Avenue PRV, which is understood to be the lag PRV in the zone. For calibration analyses, this PRV was closed, and was initialized as closed in the model accompanying this technical memorandum. It is recommended that this PRV be inspected in the field for proper operation before it is opened in the model for future analyses.
  - c. Flow Test Nos. 141 through 143: The 47<sup>th</sup> Avenue and SW 316<sup>th</sup> PRV station was oriented the wrong way in the hydraulic model. The direction of Pipes M-0061609 and M-0064331 were reversed.
  - d. Flow Test Nos. 127 through 129: When only the Indian Hills 2 BPS is operational (i.e., all pumps at the Indian Hills 1 BPS are off), the hydraulic grade of the 649 Indian Hills Zone decreases 16 feet, from 649 to 633 feet. Controls were added to the pressure sustaining valve in the Indian Hills 2 BPS to reflect this condition. Additionally, the Indian Hills 1 and Indian Hills 2 BPSs were cycling on and off during these flow tests. Based on the SCADA data provided by Tacoma Water, the status of each of these BPSs during these flow tests is shown in **Table 4**.

		ing i lon i ooung
Flow Test No.	Indian Hills 1 BPS	Indian Hills 2 BPS
127 Static	OFF	ON
127 Low and High Flows	ON	ON
128 Static	ON	OFF
128 Low and High Flows	ON	ON
129 Static	ON	ON
129 Low and High Flows	ON	ON

Table 4: Indian Hills 1 and 2 BPSs Status During Flow Testing

These BPSs were initialized in the model as shown in **Table 4** for the calibration analyses. When the Indian Hills 1 BPS is operational, only Pump 3 is on in the hydraulic model, which has controls to be modeled as a single VFD or to reflect the combined pumping capacity of Indian Hills 1 BPS Pumps 1 through 3. When the Indian Hills 2 BPS is operational, only Pump 4 was on in the hydraulic model. Following completion of the calibration analyses, Indian Hills 1 BPS Pump 3, and Indian Hills 2 BPS Pump 4 were initialized as active in the model accompanying this technical memorandum. All other pumps in these BPSs were closed.

## CONCLUSION

## STEADY-STATE CALIBRATION CONCLUSION

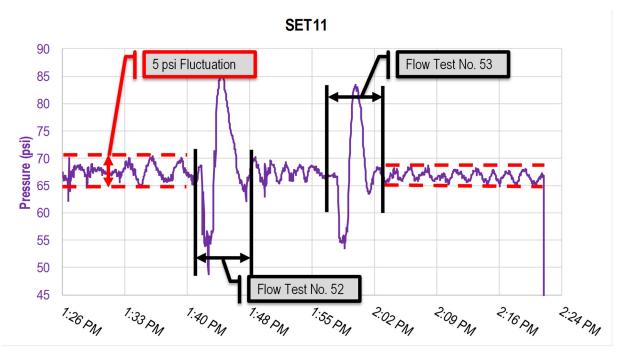
The steady-state model is calibrated within industry-recognized guidelines for the majority of the Tacoma Water distribution system. Over 91 percent of field-measured static pressures systemwide are within plus or minus 2 psi of the static pressures calculated in the hydraulic model, which meets the ECAC guideline for static pressure measurements. All 100 percent of the dynamic tests performed in the system resulted in the headloss measured in the field being within 5 to 10 feet of hydraulic grade (4.33 psi) of the headloss calculated by the hydraulic model. This level of accuracy results in all 275 tests meeting the Walski calibration guideline of the model predicting the hydraulic grade line to within 5 to 10 feet at model calibration points during peak demands, such as fire flows.

The only area not meeting industry-recognized guidelines include the static pressure measurements in the non-478 High Zone pressure zones in west and northwest Tacoma. In this area, 76 percent of the field-measured static pressures systemwide are within plus or minus 2 psi of the static pressures calculated in the hydraulic model. Many of the pressure zones within this operating area are served by PRV stations and include steep slopes adjacent to Puget Sound. Elevations in areas with steep slopes are more susceptible to inaccuracies because significant elevation changes can occur with slight changes to the X and Y coordinates of a junction or valve. Over 90 percent of field-measured static pressures in these zones are within plus or minus 3 psi of the static pressures calculated in the hydraulic model.

## **OTHER OBSERVATIONS**

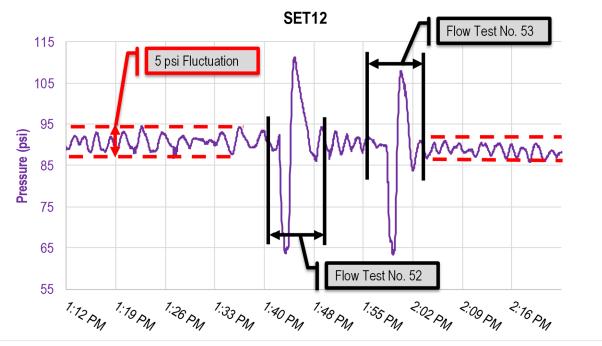
## 520 SE Tacoma Zone

The 520 SE Tacoma Zone is a closed zone served by a PRV station. Pressure transducers indicate that a pressure fluctuation of approximately 5 psi is occurring every 1 to 2 minutes in the 520 SE Tacoma Zone during static conditions, as shown in **Charts 1** and **2**. The location of these transducers, SET11 and SET12, are shown in **Figure 5**. It is recommended that the setpoint and operation of the PRV station serving the 520 SE Tacoma Zone be evaluated to confirm proper operation and possibly reduce the pressure fluctuation that was observed to be occurring during static conditions.



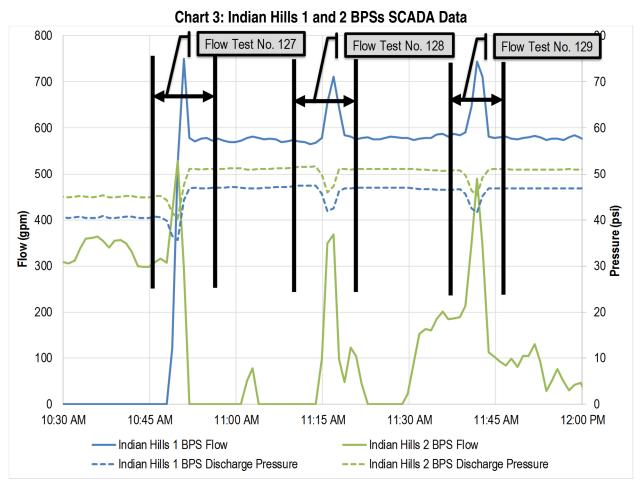
**Chart 1: SET11 Pressure Transducer Data** 

Chart 2: SET12 Pressure Transducer Data



## 649 Indian Hills Zone

As described in the **NE Tacoma Area** calibration summary, the 649 Indian Hills Zone is a closed zone served by two BPSs, and the hydraulic grade of the pressure zone fluctuates depending on the supply rate of the two BPSs. The SCADA data from the day of field testing for Flow Test Nos. 127, 128, and 129 is shown in **Chart 3** for each BPS. As shown in **Chart 3**, the pressure during non-flow test periods ranged from approximately 45 to 51 psi at the Indian Hills 1 BPS, and ranged from approximately 40 to 47 psi at the Indian Hills 2 BPS. Additionally, during Flow Test Nos. 127, 128, and 129, the pressure at the Indian Hills 1 and 2 BPSs decreased by approximately 5 psi compared to the pressure during non-flow test periods. This decrease in pressure indicates that additional pumps were not called to operate, and the pumps that were operating were moving "to the right" on the pump curves to meet the normal demand and hydrant demand in the zone while reducing the hydraulic grade of the zone. Depending on any building-specific fire flow or level-of-service requirements in the 649 Indian Hills Zone, it may be necessary to adjust the pump setpoints to allow increased flow to be provided from the Indian Hills 1 and 2 BPSs without reducing the hydraulic grade of the zone.



## **NEXT STEPS**

## 346 Portland Avenue Zone Investigation

It is recommended that Tacoma Water confirm the pressure setpoint of the three active PRV stations that serve the 346 Portland Avenue Zone and confirm that each PRV is able to operate as intended. Additionally, it is recommended that Tacoma Water confirm that the three PRV stations that are inactive in the hydraulic model (E 34<sup>th</sup> Street and Fairbanks Street PRV, the E 34<sup>th</sup> Street and N Street PRV, and the E Harrison Street and N Street PRV) be confirmed to not be active in the water system.

During a December 13, 2017 meeting, Tacoma Water indicated that a recent emergency event occurred in this zone and that PRV setpoints or initial statuses may have been adjusted during the event and then not returned to the setpoints or statuses that are initialized in the hydraulic model. Following an investigation of the PRV setpoints and statuses, approximately two additional flow tests may be recommended by RH2 to provide additional data to allow the flow tests in this zone to confirm that the field data and modeled results continue to meet at least one of the hydraulic model calibration criteria identified in the DOH *Water System Design Manual*.

## 581 Canyon Zone Investigation

It is recommended that Tacoma Water confirm the possibility of reverse flow occurring through the 148<sup>th</sup> Street E and Woodland PRV station, either through a check valve in the PRV station or through the PRV itself. A check valve was added to this PRV station in the model accompanying this technical memorandum as the analyses performed for Test No. 34 indicate that reverse flow was likely occurring through this PRV station during the field flow test.

### West and Northwest Tacoma Static Pressures

Many of the pressure zones in west and northwest Tacoma that are adjacent to the 478 High Zone have field-measured static pressures that are within plus or minus 3 psi of the modeled static pressures, but not within the ECAC guideline of plus or minus 2 psi. Some of these zones include steep slopes adjacent to Puget Sound. Elevations in areas with steep slopes are more susceptible to inaccuracies because significant elevation changes can occur with slight changes to the X and Y coordinates of a junction or valve. Tacoma Water has indicated that a high-resolution digital elevation model or similar representation of the water system's terrain is planned to be created in 2018. It is recommended that the hydraulic model elevations in these pressure zones be compared and updated with the forthcoming elevation data to determine if the updated elevation data results in at least 90 percent of the static pressure measurements and calculations are not within plus or minus 2 psi, it is recommended that the setpoints of the PRV stations serving these pressure zones be confirmed by Tacoma Water, and that the elevation of the PRV stations be field-surveyed and updated in the hydraulic model.

As elevation data is confirmed or updated in the hydraulic model, it is recommended that the source of the elevation data be added to each node, as well as the date that the elevation was confirmed or updated. One suggested method to track the source and date of the elevation

updates is to add editable fields to each node to be updated as elevations are confirmed or updated.

## **Facility Setpoint Confirmation**

It is recommended that Tacoma Water review the facility setpoint adjustments presented in this technical memorandum and compare these setpoints with the actual field setpoints. In some cases, it may be necessary to visit a facility and open a hydrant or otherwise temporarily adjust the setpoint of another facility to be able to confirm the model's setpoint assumptions.

## **Extended Period Simulation Calibration**

The model accompanying this technical memorandum is calibrated for steady state scenarios, which provides instantaneous flow and pressure results based on user-defined initial conditions (reservoir water levels, pump status, demands, etc.). Additional confirmation of the Tacoma Water hydraulic model's accuracy can occur if the model is calibrated for extended period simulation (EPS) scenarios, which would allow Tacoma Water to better analyze the operation of each facility over a period of time, including (but not limited to) the ability to analyze pump cycles and runtimes, reservoir water levels, diurnal demand cures, and water age.

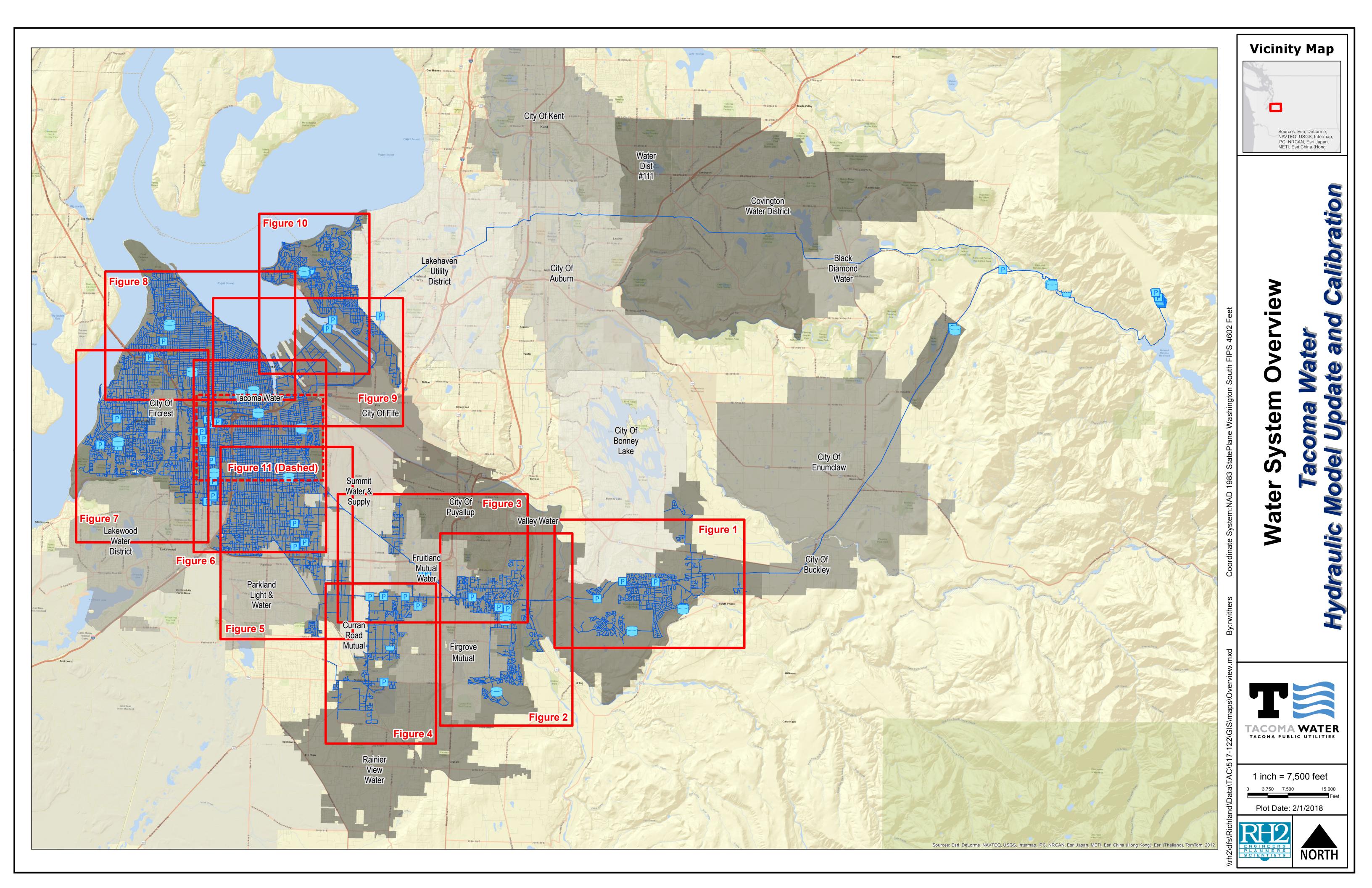
## ATTACHMENTS

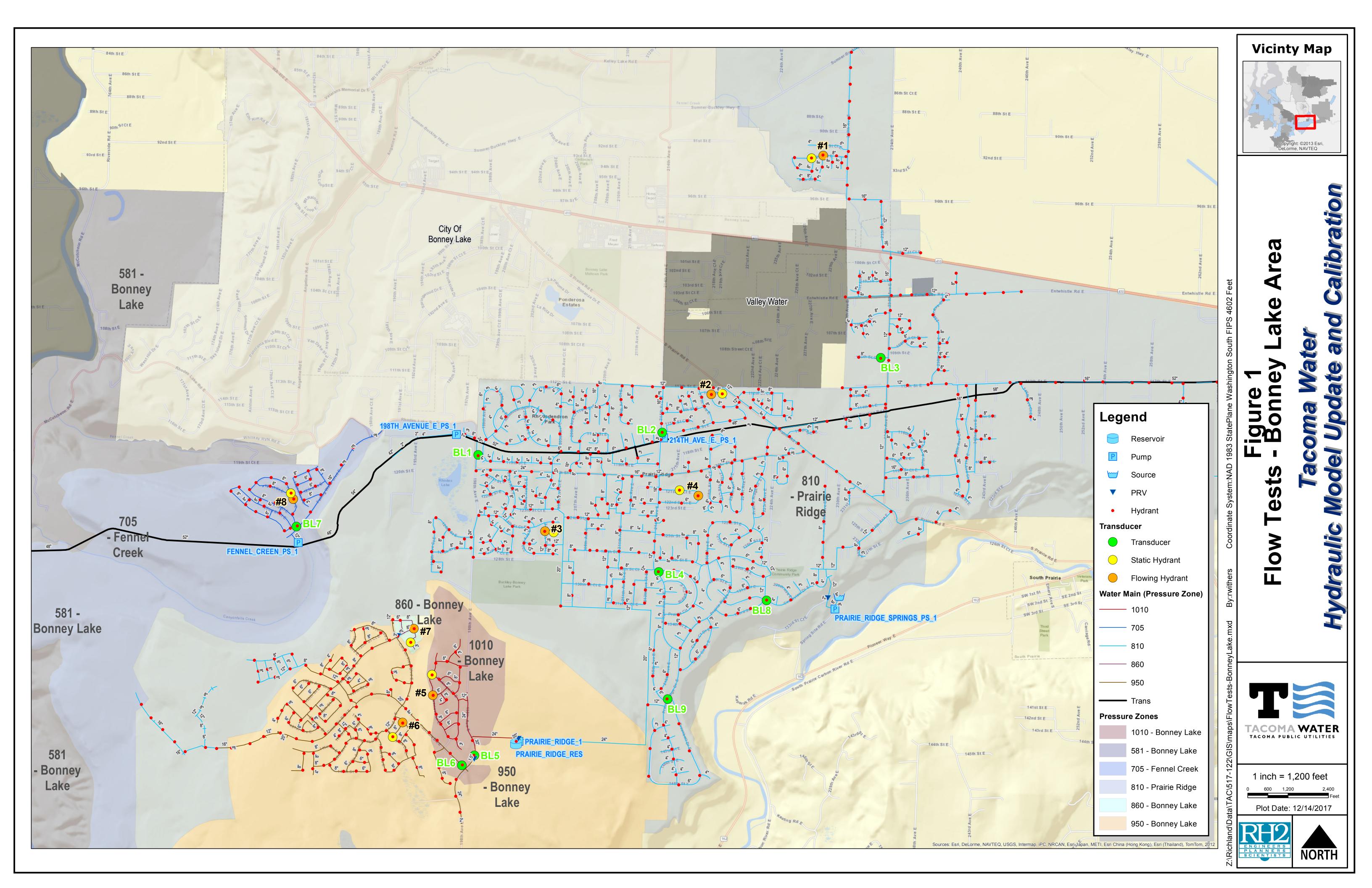
Water System Overview Figure (Figures 1 through 11 index)

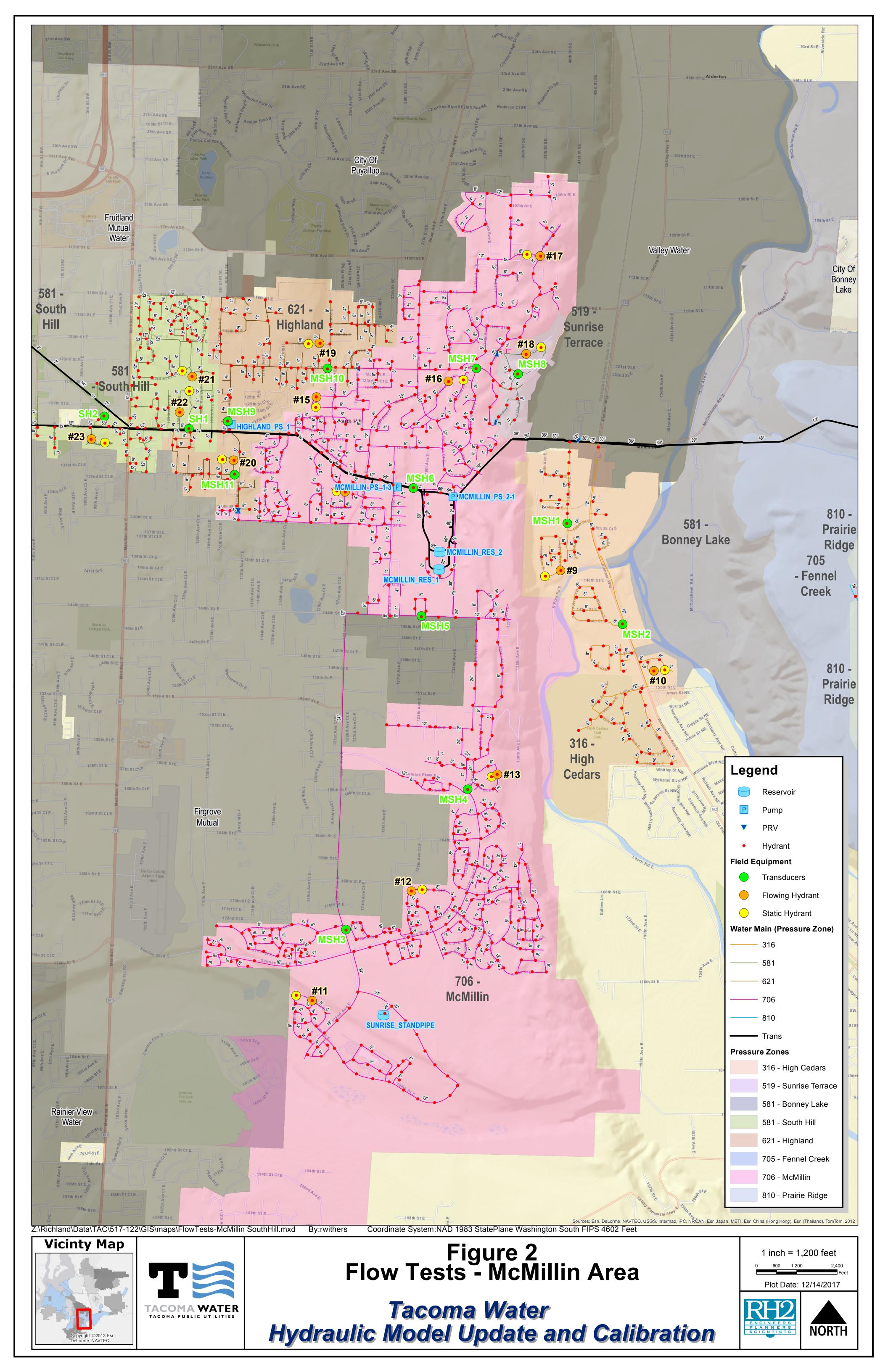
Figures 1 through 11: Field Test Locations

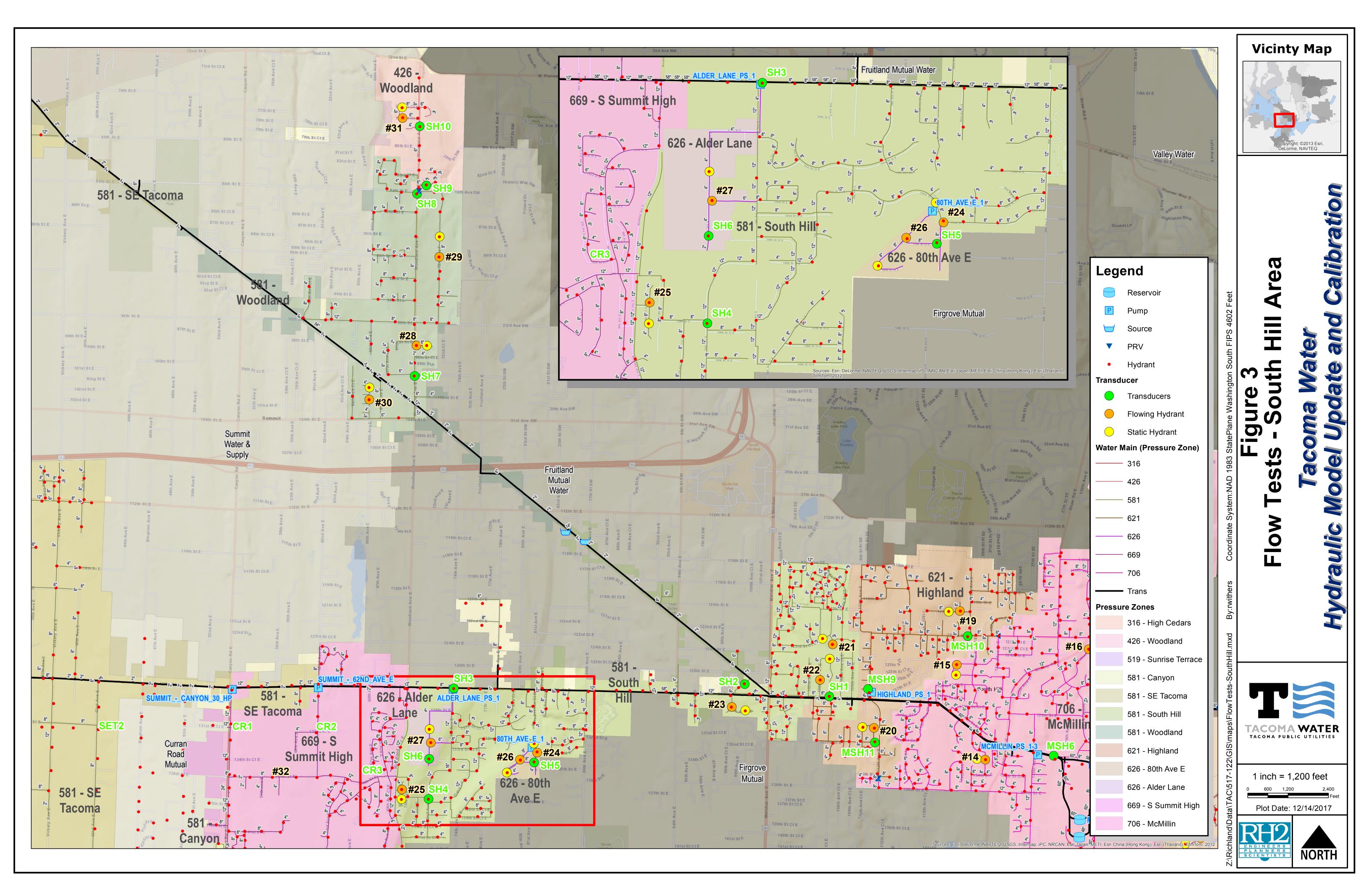
Hydraulic Model Calibration Data

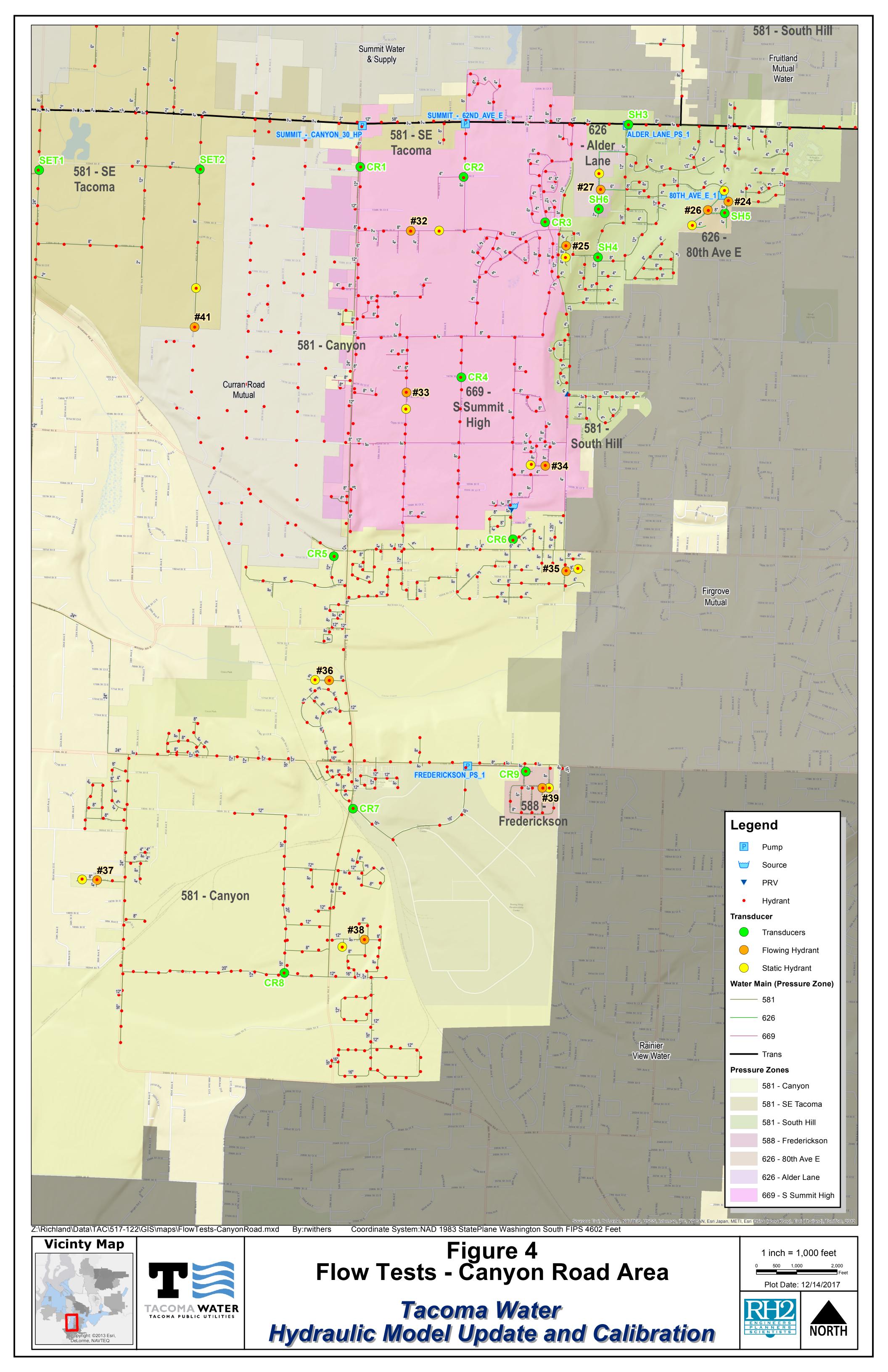
# **FIGURES**

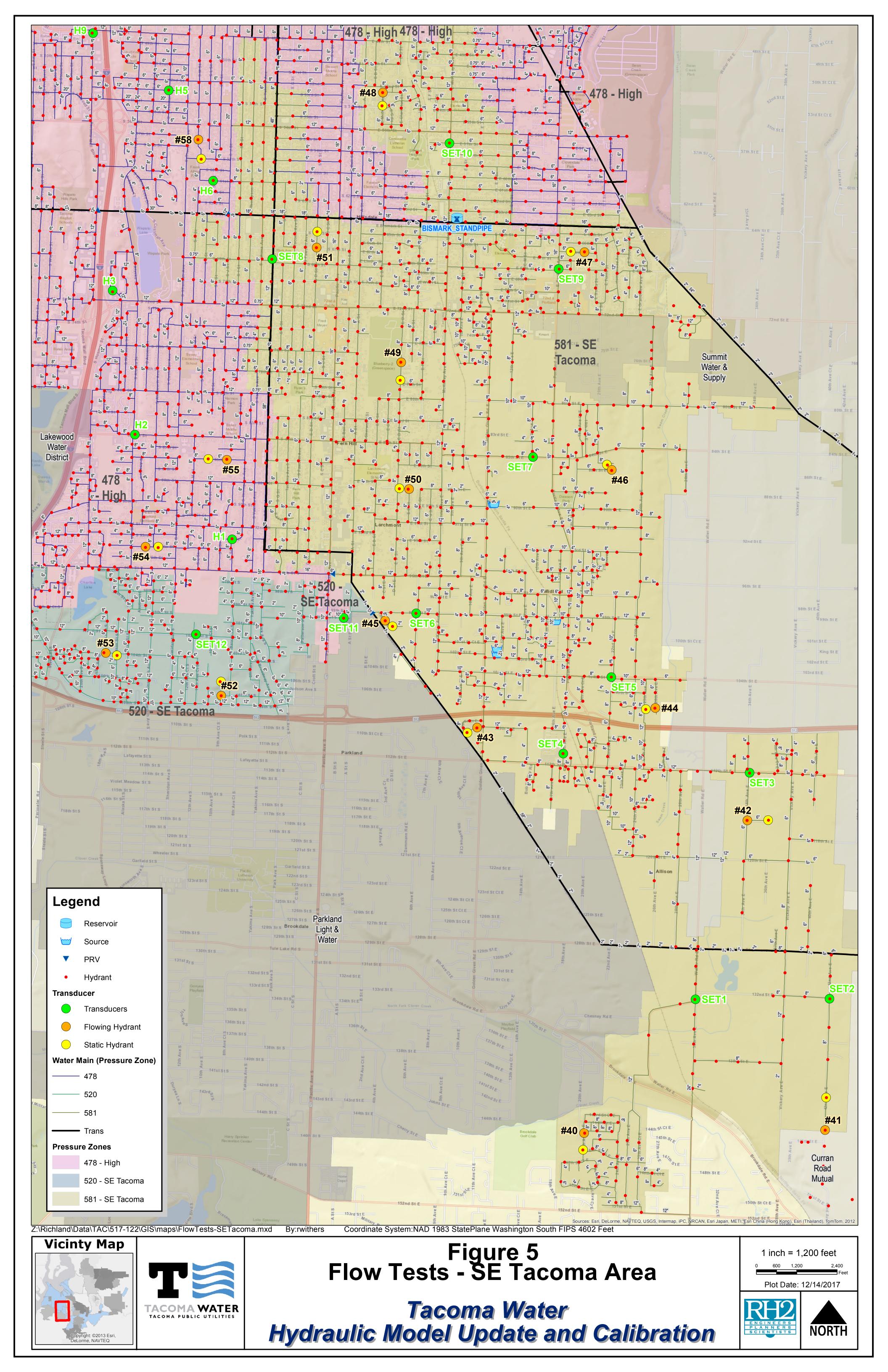


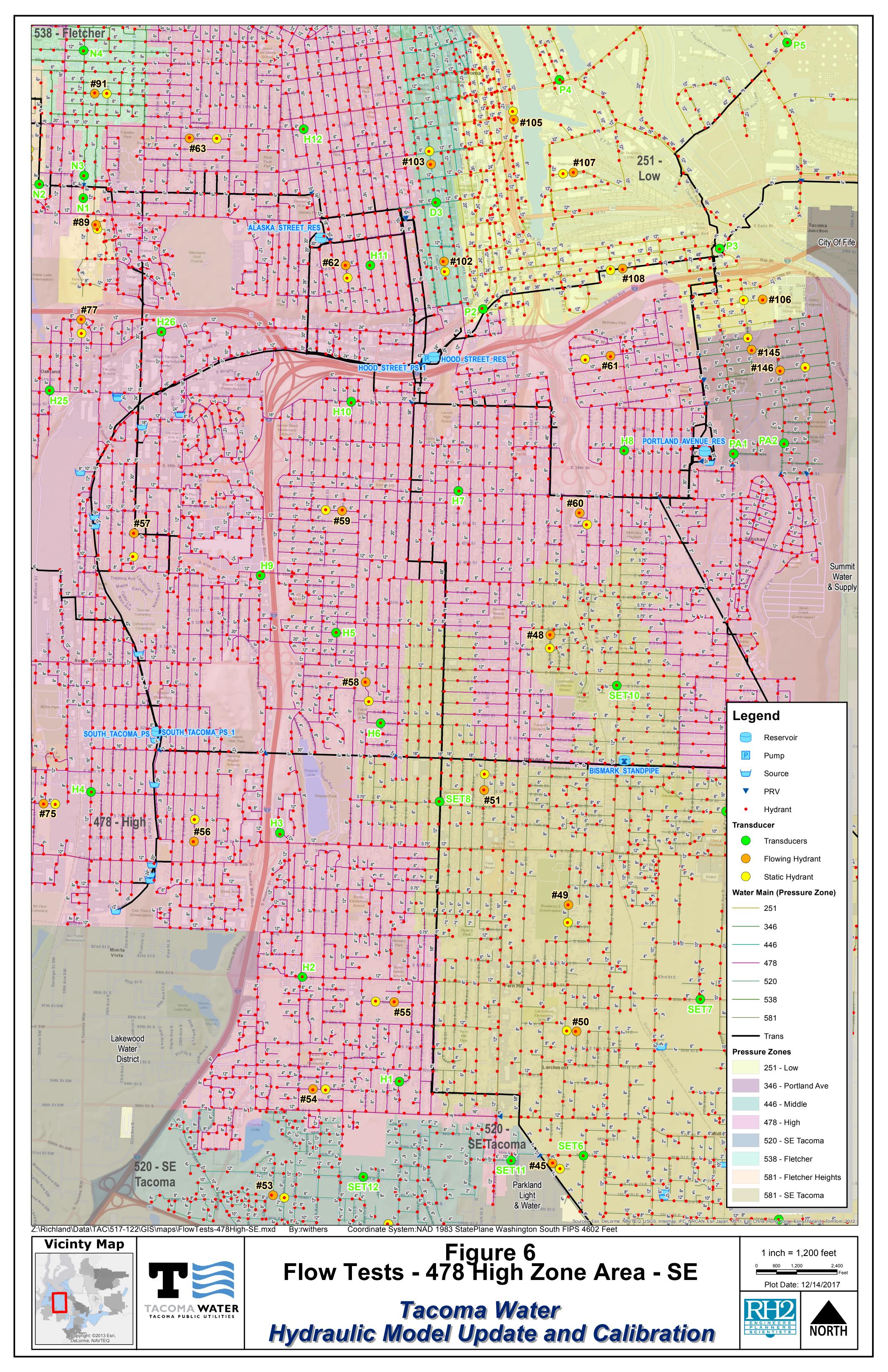


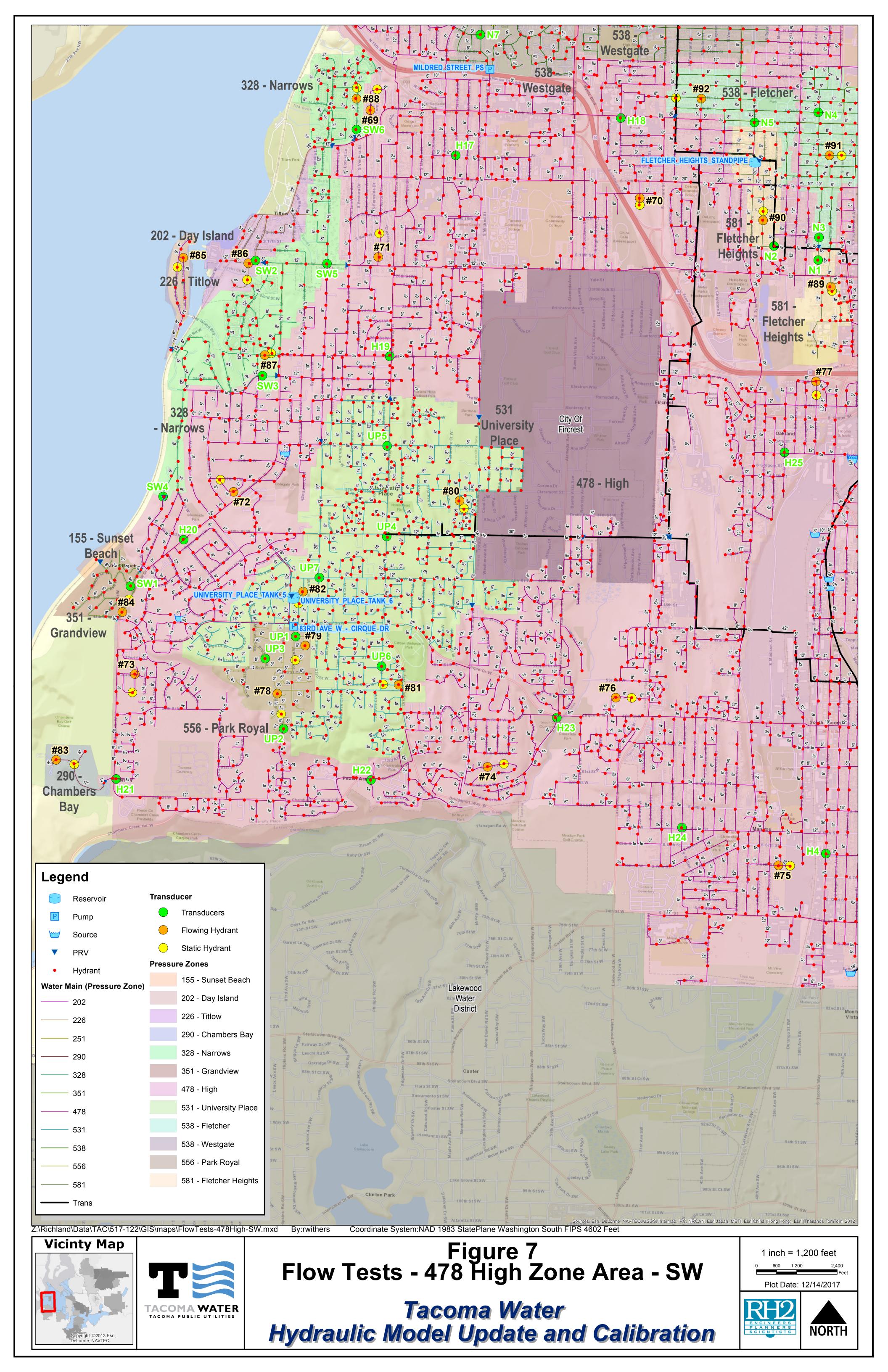


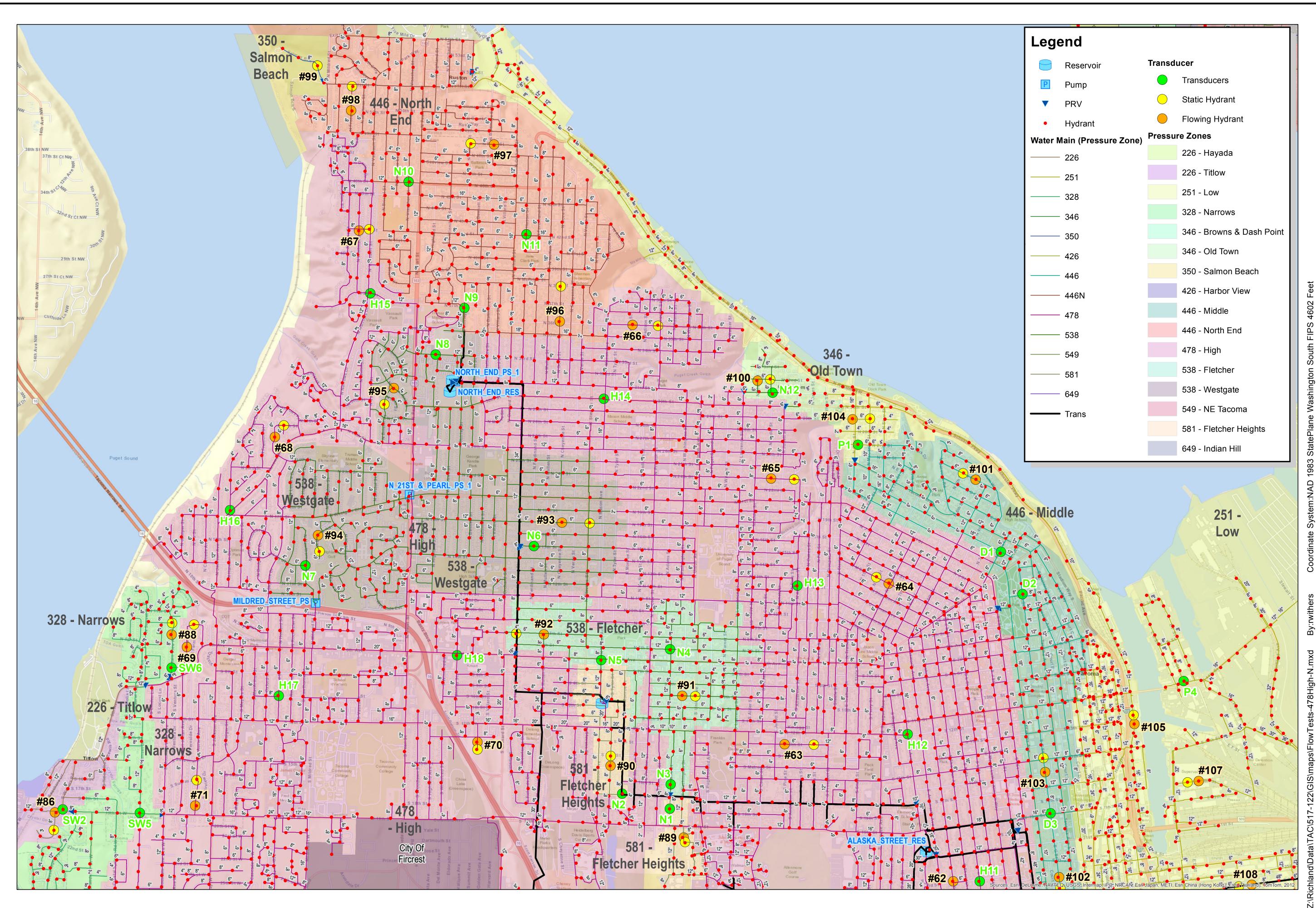




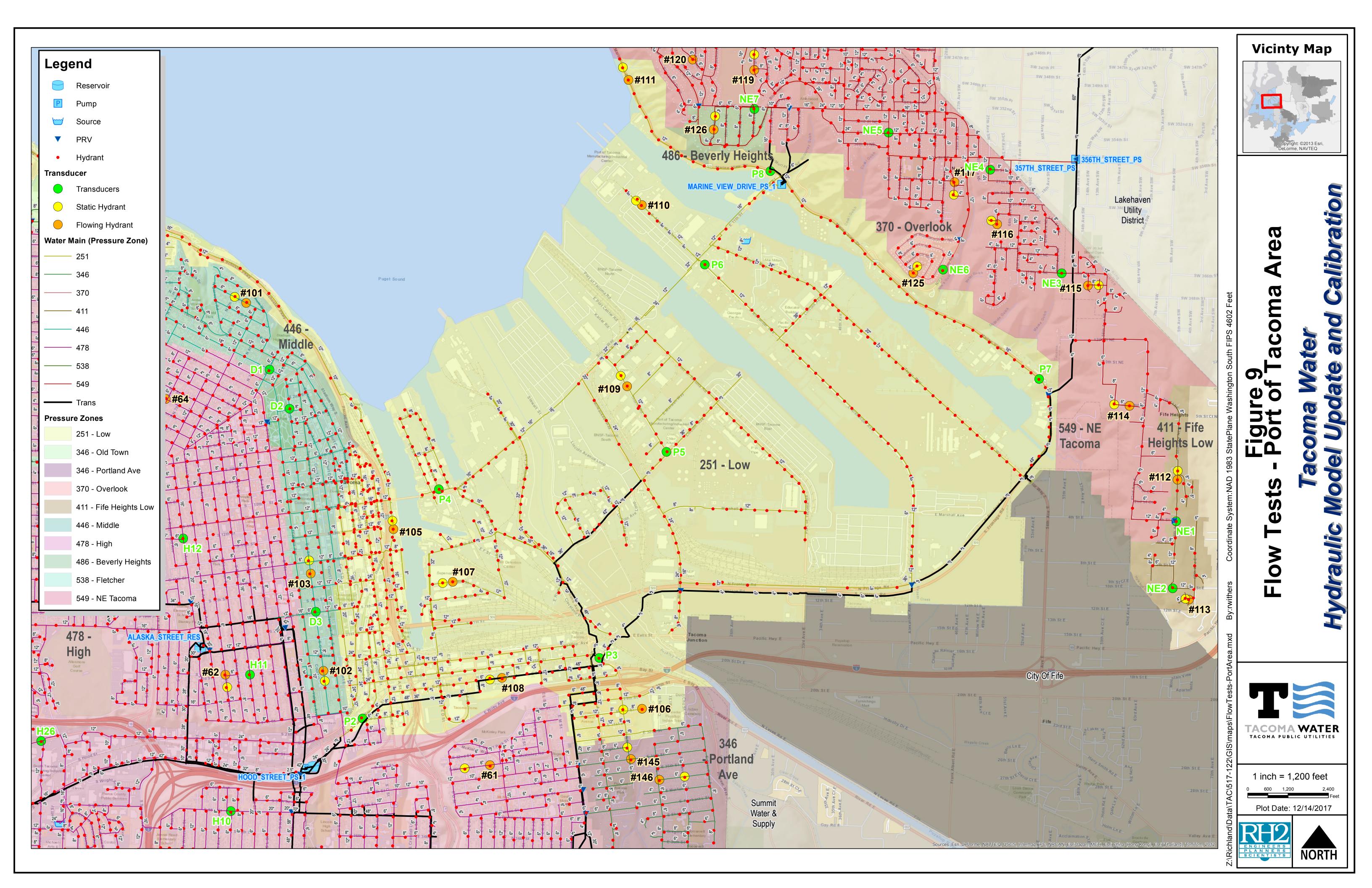


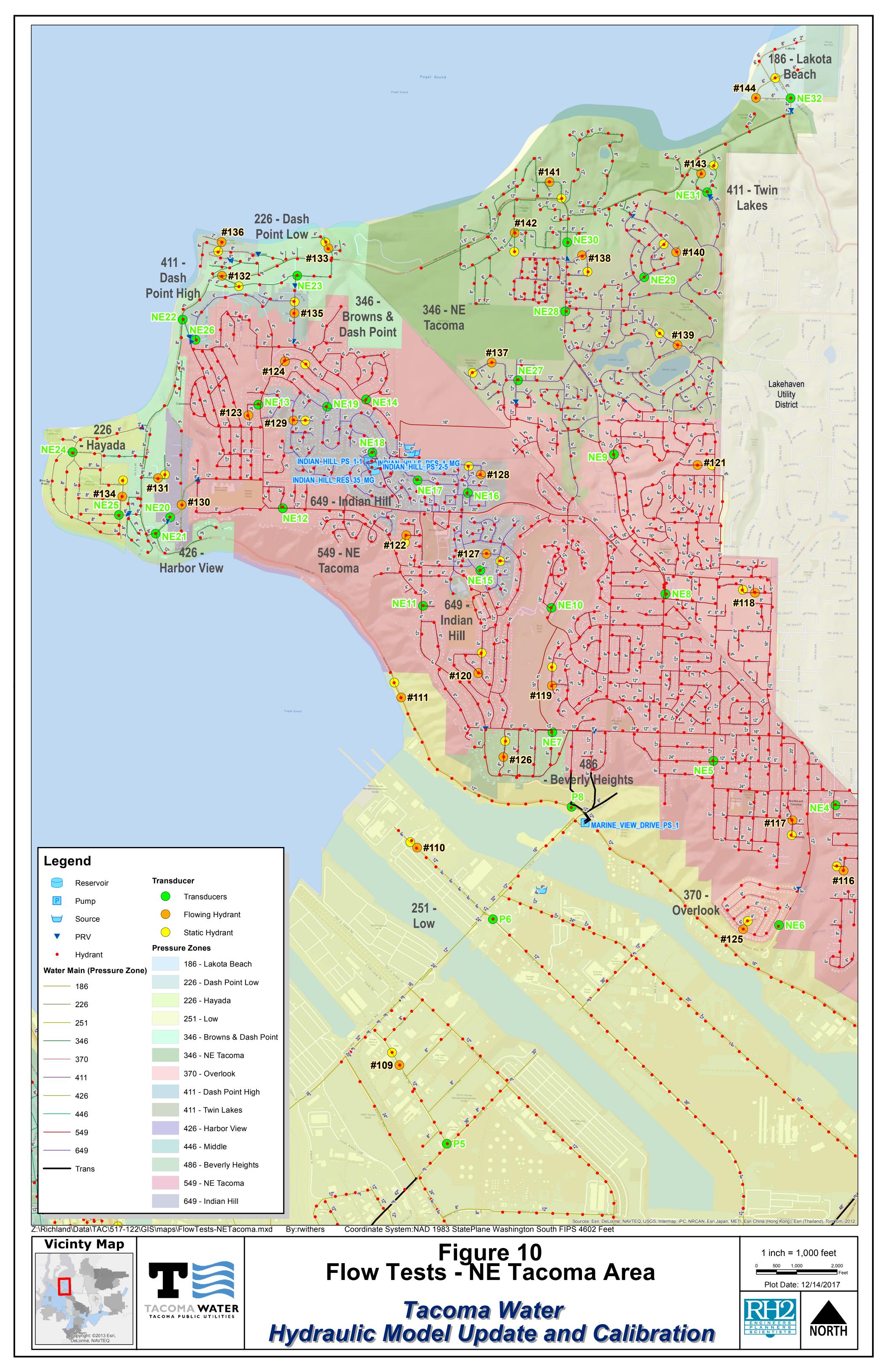


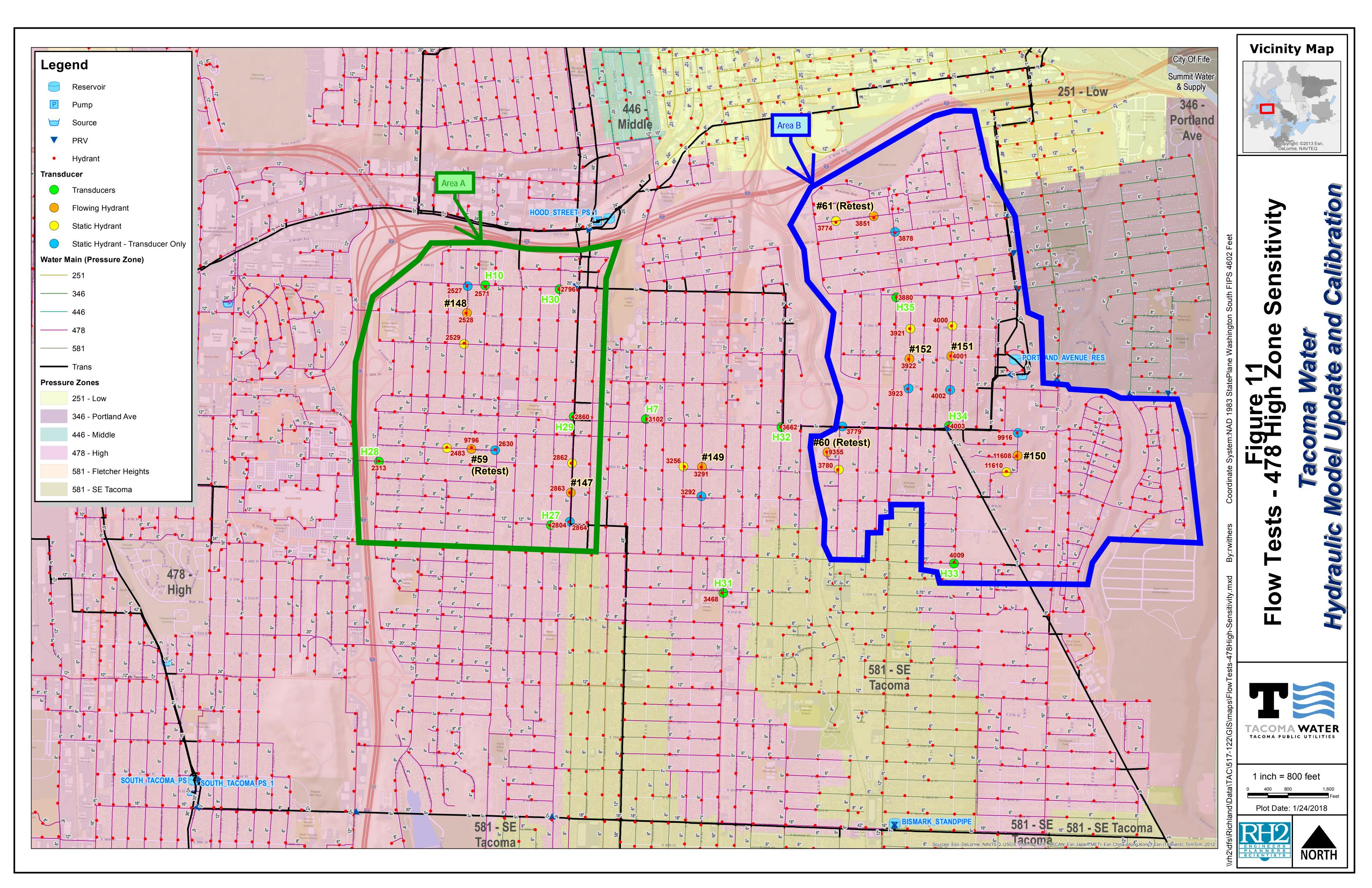












# **DATA TABLES**

### Tacoma Water Hydraulic Model Calibration Data Bonney Lake Operating Area

			1					t Flow Tested	1				Pressure Meas							1					
Test No.	Pre HGL (feet)	essure Zone Descriptor	Figure No.	Date	Time	Duration of Test (mins)	Field N Location	easurements F1 Model Node No.	Hydrant No.	Flow Calc (gpm)	Static Pressure (psi)	Location	d Measurement R1 Model Node No.	ts Hydrant No.	R1 Static Pressure (psi)	R1 Residual Pressure (psi)	F1 Static Pressure (psi)	Model Resu R1 Static Pressure (psi)	Its R1 Residual Pressure (psi)	Field Diff Pressure F1 (psi)	Model Diff Pressure F1 (psi)	Error (per flow) (psi)	Error (per site) (psi)	Static P Diff (Flow Hyd) (psi)	Static P Diff (Static Hyd) (psi)
1	810 810	Prairie Ridge	1	10/2/2017	1:55 PM 1:57 PM	1 3	92nd Street at intersection with 227th Avenue (Catch Basin)	J47468	9520	939 1,504	93	[West of Flow Hydrant] 92nd Street at intersection with 226th Avenue	J47484	9522	99	88 65	92.2	97.4	83.7 66.1	11.0 34.0	13.7 31.3	2.7 -2.7	0.0	0.8	1.6
2	810	Prairie Ridge	1	10/2/2017	2:27 PM	2	218th Avenue at intersection with 113th Street	J49604	7536	791	60	[East of Flow Hydrant]	J92676	7537	60	57	59.8	58.9	57.1	3.0	1.8	-1.2	-1.0	0.2	1.1
	810				2:29 PM	3	(Catch Basin)			1,369		Approx. 21816 113th Street				55			54.7	5.0	4.2	-0.8			
3	810	Prairie Ridge	1	10/3/2017	9:18 AM	1	204th Avenue Ct at intersection with 125th Street Ct	J50272	7474	809	66	[East of Flow Hydrant]	J100742	7476	63	60	66.9	63.3	61.4	3.0	1.9	-1.1	-0.9	-0.8	-0.3
-	810				9:19 AM	2	(Catch Basin)			1,489		125th Street Ct cul-de-sac				57			58.0	6.0	5.3	-0.7			
4	810	Prairie Ridge	1	10/3/2017	10:38 AM	1	217th Avenue Ct at intersection with 121st Street Ct	J49344	6936	843	73	[West of Flow Hydrant]	J49276	6935	74	67	74.2	73.3	67.0	7.0	6.3	-0.7	1.1	-1.2	0.7
-	810	Traine Tildge		10/0/2017	10:39 AM	2	(Catch Basin)	040044	0000	1,451	70	121st Street Ct (Mid-block)	043270	0000	/4	62	74.2	70.0	58.3	12.0	15.0	3.0		1.2	0.7
5	1010	Bonnev Lake	1	10/2/2017	10:27 AM	2	Intersection of Overlook Drive and 140th Street	J111950	12327	413	62	[North of Flow Hydrant] Intersection of Overlook Drive and	J111994	12324	61	55	60.8	61.7	59.4	6.0	2.3	-3.7	-1.3	1.2	-0.7
5	1010	Bonney Eake		10/2/2017	10:30 AM	3	(Catch Basin)	0111350	12021	962	02	Parkview Drive	0111334	12024	01	52	00.0	01.7	51.6	9.0	10.1	1.1	1.0	1.2	0.7
6	950	Bonnev Lake	1	10/2/2017	10:59 AM	2	Intersection of 193rd Avenue and 143rd Street	J90324	11707	1,012	99	[Southwest of Flow Hydrant] Intersection of 143rd Street and	J47812	11708	98	91	101.3	98.1	92.0	7.0	6.1	-0.9	-0.7	-2.3	-0.1
0	950	Bonney Eake		10/2/2017	11:03 AM	4	(Catch Basin)	000024	11707	1,736	55	Knoll Park Drive	04/012	11700	50	85	101.0	50.1	85.6	13.0	12.5	-0.5	0.7	2.0	0.1
7	860	Bonnev Lake	1	10/2/2017	11:29 AM	2	Intersection of Village Court and 193rd Avenue	J48166	11715	754	63	[Southeast of Flow Hydrant]	J48206	11716	63	56	63.5	60.5	53.6	6.5	6.9	0.4	0.8	-0.5	2.0
	860	Donnoy Eake		10,22017	11:31 AM	3	(Catch Basin)	0.0100		1,141	00	193rd Avenue cul-de-sac	0.0200		50	45	00.0	00.0	41.9	17.5	18.7	1.2	5.0	5.5	2.0
8	705	Fennel Creek	1	10/2/2017	12:15 PM	1	Approx. 12110 181st Avenue	J51408	9967	843	84	[Northwest of Flow Hydrant] 181st Avenue at intersection with	J51410	9968	80	60	84.4	81.2		20.0			2.1	-0.4	-1.2
0	705	I GINGI GIEEK	'	10/2/2017	12:17 PM	2	(Catch Basin)	001400	5507	1,736	04	121st Street	001410	5500	30	68	04.4	01.2	67.1	12.0	14.1	2.1	2.1	0.4	1.2

### Tacoma Water Hydraulic Model Calibration Data McMillin Operating Area

			Hydrant Flow Tested Hydrant for Pres							Pressure Mea	surements						1								
								leasurements	u				Measuremen					Model Resu	ilts	Field Diff	Model Diff			Static P	Static P
						Duration		F1 Model		Flow	Static		R1 Model			R1 Residual			R1 Residual	Pressure	Pressure	Error	Error	Diff	Diff
Test No.	Pre HGL (feet)	essure Zone Descriptor	Figure No.	Date	Time	of Test (mins)	Location	Node No.	Hydrant No.	Calc (gpm)	Pressure (psi)	Location	Node No.	Hydrant No.	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	F1 (psi)	F1 (psi)	(per flow) (psi)	(per site) (psi)	(Flow Hyd) (psi)	(Static Hyd) (psi)
NO.	316	Descriptor	Figure No.	Date	10:22 AM	1	Intersection of 141st Avenue and	INU.	INO.	(gpiii) 893	(psi)	[West of Flow Hydrant]	INU.	INU.	(psi)	(psi) 67	(psi)	(psi)	(psi) 67.0	(psi) 11.0	(psi) 12.7	(psi) 1.7	(psi)	(psi)	(psi)
9	316	High Cedars	2	10/5/2017	10:23 AM	1	140th Street (Catch Basin)	J84952	8620	1,404	81	Intersection of 140th Street and 139th Avenue	J84922	8633	78	58	81.6	79.7	59.8	20.0	19.9	-0.1	0.8	-0.6	-1.7
	316				9:32 AM	1	148th Street just east of intersection with 148th Avenue			843		[East of Flow Hydrant]				75			67.4	9.0	12.7	3.7			
10	316	High Cedars	2	10/5/2017	9:33 AM	2	Court (Catch Basin)	J90226	7134	1,391	79	Intersection of 148th Street and 151st Avenue	J47906	7132	84	53	78.8	80.0	51.8	31.0	28.2	-2.8	0.4	0.2	4.0
	706				1:35 PM	1	181st Street at intersection with			939		[West of Flow Hydrant]				87			86.6	2.5	2.3	-0.3			
11	706	McMillin	2	10/4/2017	1:37 PM	2	120th Avenue (Catch Basin)	J52640	9456	1,801	88	Intersection of 181st Street and 118th Avenue Court	J52600	9455	90	84	87.5	88.8	81.9	5.5	6.9	1.4	0.6	0.5	0.7
12	706	McMillin	2	SKIPPED			168th Street Court at intersection with 128th Avenue	J53388	7653			[East of Flow Hydrant]	J53402	7654											
12	706	MCMIIIIN	2	SKIPPED			(Catch Basin)	J23388	/653			Approx. 12900 168th Street Court	J53402	/654											
13	706	McMillin	2	10/4/2017	2:51 PM	1	Intersection of 136th Avenue Court and 158th Street Court	J90840	7865	754	57	[West of Flow Hydrant] Mid-block on 158th Street Court,	J54526	7866	51	49	55.8	51.8	49.2	2.5	2.7	0.2	-0.3	1.2	-0.8
	706	Hommin	-	10/ 11/2011	2:53 PM	2	(Catch Basin)	000010	1000	1,117	0.	towards cul-de-sac	001020	1000	0.	44	00.0	01.0	45.5	7.0	6.3	-0.7	0.0		0.0
14	706	McMillin	2	SKIPPED			On 122nd Avenue at intersection with 133rd Street	J93066	8299			[West of Flow Hydrant] Mid-block on 133rd Street.	J57244	8300											
	706		_				(Catch Basin)					towards cul-de-sac													
15	706	McMillin	2	10/4/2017	10:05 AM	2	At intersection of 124th Street Court and 118th Avenue Court	J58376	5389	843	73	[South of Flow Hydrant] On 125th Street Court, just east of	J58370	5390	69	66	71.9	70.3	63.8	3.0	6.4	3.4	0.1	1.1	-1.3
	706				10:07 AM	3	(Catch Basin)			1,411		118th Avenue Court				50			54.6	18.9	15.6	-3.3			
16	706	McMillin	2	10/4/2017	11:35 AM	2	Approx. 12360 Tatoosh Road- In front of Stratton Park	J58128	5478	893	78	[Northeast of Flow Hydrant] 132nd Avenue, just south of	J102522	5477	71	56	77.0	70.1	58.5	14.7	11.6	-3.2	-0.2	1.1	0.9
	706				11:38 AM	4	(Catch Basin)			1,347		intersection with Tatoosh Road				50			46.3	21.0	23.8	2.8			
17	706	McMillin	2	10/4/2017	9:28 AM	2	Intersection of Shawnee Road and 111th Street Court	J107294	11818	969	84	[West of Flow Hydrant] Mid-block on 111th Street Court,	J107254	11820	83	79	82.5	81.7	73.4	4.0	8.2	4.2	0.5	1.5	1.3
	706				9:31 AM	3	(Catch Basin)			1,582		towards cul-de-sac				61			62.9	22.0	18.8	-3.2			
18	519	Sunrise Terrace	2	10/3/2017	1:59 PM	1	120th Street at intersection with 137th Avenue Court	J108562	5526	600	59	[East of Flow Hydrant] 120th Street at intersection with	J108604	12303	83	65	56.5	82.7	66.5	18.0	16.3	-1.7	-1.7	2.6	0.3
	519						(Catch Basin)					138th Avenue													
19	621 621	Highland	2	10/3/2017	1:14 PM	2	Intersection of 120th Street and 119th Avenue (Catch Basin)	J103202	11729	773	72	[West of Flow Hydrant] ~300 feet west of flow hydrant on 120th Street at end of sidewalk	J58616	5393	61	47	70.4	62.2	49.6	14.0	12.6	-1.4	-1.4	1.6	-1.2
	621				 12:38 PM	2	(Catch Basin) Near intersection of 111th Avenue			754		[West of Flow Hydrant]				 48	_	_	45.9		12.9	1.9			
20	621	Highland	2	10/3/2017	12.30 F W		Court and 130th Street Court (Catch Basin)	J57054	7640		61	Near intersection of 130th Street Court and 110th Avenue Court	J57060	7639	59	40	60.9	58.8			12.5		1.9	0.1	0.2
	021		1				(outon basin)					Court and Treat Avenue Court													

### Tacoma Water Hydraulic Model Calibration Data South Hill Operating Area

	1 1						Hydrant	Flow Tested	1			Hydrant for F	Pressure Mea	surements											
							Field Me	easurements					d Measuremen					Model Resu			Model Diff			Static P	Static P
Test	Dro	sure Zone				Duration of Test		F1 Model Node	Hydrant	Flow Calc	Static Pressure		R1 Model Node	Hydrant	R1 Static Pressure	R1 Residual Pressure	F1 Static Pressure	R1 Static Pressure	R1 Residual Pressure	Pressure F1	Pressure F1	Error (per flow)	Error (per site)	Diff (Flow Hyd)	Diff (Statio Llud)
No.	HGL (feet)	Descriptor	Figure No.	Date	Time	(mins)	Location	No.	No.	(gpm)	(psi)	Location	No.	No.	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(per now) (psi)	(per site) (psi)	(FIOW Hyd) (psi)	(Static Hyd) (psi)
21	581SH	South Hill	3	10/5/2017	1:41 PM	1	On private road south/southwest of intersection of 123rd Street Court and 107th Avenue Court	J58682	5356	716	48	[North/northwest of Flow Hydrant] Intersection of 123rd Street Court	J58744	5352	40	39	47.2	41.9	40.1	1.0	1.8	0.8	1.4	0.8	-1.9
	581SH				1:42 PM	1	(Field/vacant lots)			1,118		and 106th Avenue Court				38			37.9	2.0	4.0	2.0			
22	581SH	South Hill	3	11/1/2017	11:15 AM	1		J104190	5353	631	50	[Northeast of Flow Hydrant] Intersection of 125th Street and	J58786	5357	49	48	49.9	48.3	46.9	1.0	1.4	0.4	0.1	0.1	0.7
	581SH				11:17 AM	2	(Catch Basin)			1,117		107th Avenue Court				45			44.4	4.0	3.9	-0.1			
23	581SH	South Hill	3	11/1/2017	11:36 AM	1	Intersection of 99th Avenue and 129th Street Court	J56524	5306	809	58	[East of Flow Hydrant] Mid-block on 129th Street Court,	J56504	5307	60	58	57.1	57.8	55.4	2.0	2.4	0.4	0.1	0.9	2.2
	581SH				11:38 AM	2	(Catch Basin)			1,449		towards cul-de-sac				53			50.9	7.0	6.8	-0.2			
24	581SH	South Hill	3	10/5/2017	2:59 PM	1	Intersection of 133rd Street and 81st Avenue	J99376	9747	640	50	[North of Flow Hydrant] Just north of intersection of 132nd	J56370	6418	46	45	48.9	47.4	46.0	1.0	1.4	0.4	0.6	1.1	-1.4
24	581SH	Godin nim	0	10/3/2017	3:00 PM	2	(Catch Basin)	000070	5747	1,159	50	Street Court 80th Avenue	000070	0410	40	43	40.5	47.4	43.6	3.0	3.9	0.9	0.0		1.4
25	581SH	South Hill	3	10/5/2017	12:43 PM	1	Intersection of 137th Street Ct	J88254	6881	653	43	69th Avenue Ct Dead end	J55550	6882	47	44	43.1	46.4	42.9	3.0	3.6	0.6	1.3	-0.1	0.6
25	581SH	South Hill	3	10/5/2017	12:43 PM	1	and 69th Ave Ct	J00204	0001	1,092	43	Batil Avenue Ci Dead end	155550	0002	47	40	43.1	40.4	37.5	7.0	9.0	2.0	1.5	-0.1	0.6
26	626	80th Avenue E	3	10/9/2017	11:43 AM	1	Intersection of 80th Avenue and 134th Street Court	J99384	9855	631	60	[Southwest of Flow Hydrant] Intersection of 80th Avenue and	J56352	9856	60	39	58.9	59.0	36.5	21.0	22.5	1.5	1.9	1.1	1.1
20	626	ootii Avenue E	5	10/3/2011	11:44 AM	2	(Catch Basin)	000004	5055	1,118	00	135th Street Court	000002	3030	00	36	50.5	55.0	32.6	24.0	26.3	2.3	1.5		1.1
27	626	Alder Lane	3	10/9/2017	10:57 AM	1	Approx. 13300 72nd Avenue	J56130	5240	608	70	[North of Flow Hydrant]	J56110	5269	72	41	69.7	71.0	35.9	31.0	35.1	4.1	2.7	0.3	1.0
	626				10:57 AM	1	(Grass Ditch)			846		Approx. 13120 72nd Avenue				33			30.8	39.0	40.2	1.2			
28	581W	Woodland	3	10/6/2017	10:53 AM	1	Intersection of Woodland Avenue and 97th Street Court	J59500	7923	893	69	[East of Flow Hydrant] On 97th Street Court towards	J59502	7924	77	73	68.6	76.2	71.8	4.0	4.4	0.4	0.8	0.4	0.8
20	581W	Woodiand	0	10/0/2011	10:54 AM	1	(Grass Ditch and Catch Basin to east)	000000	7520	1,432	00	dead end	000002	7524		68	00.0	70.2	66.1	9.0	10.1	1.1	0.0	0.4	0.0
29	581W	Woodland	3	10/6/2017	11:39 AM	1	Approx. 8921 72nd Avenue	J96378	7208	1,040	115	[North of Flow Hydrant] Approx. 600 feet north of flow	J51950	7207	122	106	115.6	119.7	102.2	15.5	17.5	2.0	0.2	-0.6	1.8
	581W		3	10,0,2017	11:41 AM	2	(Grass Ditch/Field)	000070	.200	1,812	.10	hydant on 72nd Avenue	00.000	.207	.22	73			72.8	48.5	46.9	-1.6	0.2	0.0	0
30	581W	Woodland	3	10/6/2017	9:48 AM	1	On 66th Avenue between 102nd Street and 103rd Street	J59412	5216	695	71	[North of Flow Hydrant] Intersection of 66th Avenue and	J59448	7202	71	43	70.6	71.5	42.0	28.0	29.4	1.4	1.4	0.4	-0.5
	581W		3	.5/0/2017	9:49 AM	1	(Grass field across street, and catch basin to north)	555412	5210	923		101st Street	333440	1202	,,	23	70.0	71.5	22.0	48.0	49.5	1.5	1.4	0.4	0.0
31	426	Woodland	3	10/6/2017	1:18 PM	1	77th Street Court cul-de-sac	J52246	7562	608	72	[North of Flow Hydrant]	J52260	9392	84	50	72.7	83.3	48.1	34.0	35.2	1.2	1.2	-0.7	0.7
31	426	*****	5	10/0/2017			(Catch Basin to east)	002240	, 302		, 2	5th Avenue Court cul-de-sac	032200	3032	54		12.1	00.0					1.2	0.7	0.7

### Tacoma Water Hydraulic Model Calibration Data Canyon Operating Area

	1					Hydran	Flow Tested	d			Hydrant for Pr	ressure Meas	urements											
						Field M	leasurements				Field	Measurements	S				Model Resu			Model Diff	_	_	Static P	Static P
Test P No. HGL (feet	ressure Zone ) Descriptor	Figure No.	Date	Time	Duration of Test (mins)	Location	F1 Model Node No.	Hydrant No.	Flow Calc (gpm)	Static Pressure (psi)	Location	R1 Model Node No.	Hydrant No.	R1 Static Pressure (psi)	R1 Residual Pressure (psi)	F1 Static Pressure (psi)	R1 Static Pressure (psi)	R1 Residual Pressure (psi)	F1 (psi)	Pressure F1 (psi)	Error (per flow) (psi)	Error (per site) (psi)	Diff (Flow Hyd) (psi)	Diff (Static Hyd) (psi)
32 669	S Summit High	4	10/9/2017	10:25 AM	1	136th Avenue between 57th Avenue and 58th Avenue (Catch Basin/Ditch)	J55654	5181	773	89	[East of Flow Hydrant] Approx. 6000 136th Street (700 feet east of flow hydrant)	J55656	5194	87	54	88.3	85.7	54.4	33.0	31.3	-1.7	-1.7	0.8	1.3
669				10:27 AM	2	(			1,186		(700 leet east of llow hydrant)				43			43.4	44.0	42.3	-1.7			
669	S Summit High	4	10/9/2017	1:25 PM	1	58th Avenue just north of intersection with 148th Street	J88120	6327	809	95	[South of Flow Hydrant] On 58th Avenue approximately 400	J88118	6326	92	72	93.3	93.3	77.7	19.8	15.6	-4.2	-1.6	1.7	-1.3
669				1:25 PM	1	Court (Grass Ditch)			1,283		feet south of flow hydrant				47			47.4	45.0	45.9	0.9			
669	S Summit High	4	10/9/2017	1:50 PM	1	Intersection of 68th Avenue Court and 153rd Street Court	J55156	9359	653	88	[West of Flow Hydrant]	J87984	9360	93	46	89.3	91.9	43.0	47.0	48.9	1.9	2.2	-1.3	1.1
669				1:51 PM	2	(Catch Basin)			1,039		153rd Street Court cul-de-sac				35			31.3	58.0	60.6	2.6			
581C	Canvon	4	10/11/2017	1:00 PM	1	161st Street Ct and 70th Ave E	J53318	7987	640	75	[East of Flow Hydrant]	J53308	7988	77	60	73.6	77.1	61.6	17.0	15.5	-1.5	0.4	1.4	0.0
581C	Guiljon		10/11/2017	1:01 PM	1		000010	1001	1,181		161st Street Ct cul-de-sac	000000	1000		48	70.0		45.6	29.0	31.4	2.4	0.1		0.0
581C	Canvon	4	10/11/2017	11:19 AM	1	170th Street, mid-block west of intersection with Canvon Road	J89042	7952	954	98	[West of Flow Hydrant]	J61140	7953	99	91	97.7	97.8	93.3	8.0	4.5	-3.5	-1.0	0.3	1.2
581C	Canyon	4	10/11/2017	11:19 AM	1	(Catch Basin)	J09042	7952	1,801	90	170th Street cul-de-sac	J61140	7953	99	87	97.7	97.0	84.4	12.0	13.4	1.4	-1.0	0.3	1.2
581C	Canvon	4	10/11/2017	10:47 AM	1	Intersection of 185th Street Courth and 36th Avenue	J60942	9567	939	81	[West of Flow Hydrant]	J60936	9568	82	76	80.6	81.4	74.7	6.0	6.8	0.8	0.5	0.4	0.6
581C				10:48 AM	1	(Catch Basin)			1,542		185th Street Court cul-de-sac				66			65.3	16.0	16.2	0.1			
581C	0		10/11/2017	9:57 AM	1	189th Street, east of intersection with Canyon Road and in front of	150770	7400	954	81	[West of Flow Hydrant] In parking lot west of flow hydrant (In	105004			81	70.7	70.0	77.0	-1.0	1.6	2.6			
38 581C	Canyon	4	10/11/2017	9:59 AM	3	Hall Forest Products (Catch Basin)	J52778	7493	1,703	81	Rainier View Water Co. and Richardson Well Drilling parking lot)	J95884	11619	80	80	79.7	78.6	75.2	0.0	3.4	3.4	3.0	1.3	1.4
588	Frederickson	4	10/26/2017	9:13 AM	1	Intersection of 69th Avenue and 177th Street Court	J52926	8525	716	60	[East of Flow Hydrant] Intersection of 177th Street Court	J52984	8524	59	47	60.9	60.2	44.4	12.0	15.8	3.8	0.7	-0.9	-1.2
588	1.1000100001		10/20/2017	9:15 AM	2	(Catch Basin)	002020	0020	1,117	00	and 69th Avenue Court	002004	0024	00	39	55.5	00.2	42.5	20.0	17.7	-2.3	0.7	0.0	

### Tacoma Water Hydraulic Model Calibration Data SE Tacoma Operating Area

-				Hydrant Flow Tested Hydrant for Pressure Measurements												1									
						Duration	Field N	feasurements F1 Model		Flow	Static	Field	Measurement R1 Model	ts	R1 Static	P1 Posidual	F1 Static	Model Resu R1 Static	Its R1 Residual	Field Diff Pressure	Model Diff Pressure	Error	Error	Static P Diff	Static P Diff
Test No.	Pr HGL (feet)	essure Zone Descriptor	Figure No.	Date	Time	of Test (mins)	Location	Node No.	Hydrant No.	Calc (gpm)	Pressure (psi)	Location	Node No.	Hydrant No.	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	F1 (psi)	F1 (psi)	(per flow) (psi)	(per site) (psi)		
40	581SE	SE Tacoma	5	10/10/2017	9:48 AM	1	Intersection of 145th Street and 20th Avenue Court	J114906	11020	1,067	102	[South of Flow Hydrant] Intersection of 20th Avenue Court	J114932	6582	105	100	103.0	103.4	98.1	5.0	5.3	0.3	-0.6	-1.0	1.6
	581SE		-		9:50 AM	1	(Catch Basin)			1,832		and 146th Street Court				90			89.9	15.0	13.6	-1.4			
41	581SE	SE Tacoma	5	10/10/2017	10:25 AM	1	Approx. 14300 42nd Avenue (Ditch)	J115190	5157	716	66	[North of Flow Hydrant] Approx. 14100 42nd Avenue	J115194	5156	65	47	67.0	65.2	48.0	18.0	17.3	-0.7	0.5	-1.0	-0.2
	581SE				10:26 AM	1				1,030						33			31.5	32.0	33.7	1.7			
42	581SE 581SE	SE Tacoma	5	10/12/2017	9:37 AM 9:38 AM	1	Intersection of 34th Avenue and 116th Street Court (Catch Basin)	J104600	5126	716 1,039	61	[East of Flow Hydrant] 116th Street Court Dead End	J104596	7392	58	45 29	59.8	57.1	41.4 26.6	13.0 29.0	15.6 30.5	2.6 1.5	2.1	1.2	0.9
	581SE	05 T	-		1:47 PM	1	Just west of intersection of Golden Given Road E and 109th Street Ct			640		[Southwest of Flow Hydrant]				75			79.3	5.0	1.7	-3.3			
43	581SE	SE Tacoma	5	10/13/2017	1:47 PM	1	(southern entrance to townhome development)	J105388	9904	1,617	83	Within townhome development in southwest portion of development	J105392	9903	80	68	84.4	81.1	73.2	12.0	7.9	-4.1	-3.7	-1.4	-1.1
44	581SE	SE Tacoma	5	10/12/2017	10:06 AM	1	Intersection of 106th Street and 26th Avenue	J113994	8941	826	68	[West of Flow Hydrant] Intersection of 106th Street and	J113976	8940	63	60	65.4	61.1	58.0	3.0	3.1	0.1	-0.7	2.6	1.4
	581SE				10:07 AM	1	(Ditch/Catch Basin)			1,350		25th Avenue				55			54.6	8.0	6.5	-1.5			
45	581SE	SE Tacoma	5	10/12/2017	10:40 AM	1	2nd Avenue Court at entrance to cul-de-sac (south of 2nd Avenue intersection with 99th Street)	J96908	9149	876	78	[Southeast of Flow Hydrant] Approx. 10500 2nd Avenue Court,	J96852	9150	79	70	78.7	77.1	67.5	9.0	9.6	0.6	-0.1	-0.7	1.9
	581SE				10:40 AM	1	(Catch Basin)			1,407		towards dead end				58			57.0	21.0	20.1	-0.9			
46	581SE	SE Tacoma	5	10/12/2017	11:21 AM	1	On 86th Street, east of intersection with 21st Avenue	J60390	9390	876	69	[Northwest of Flow Hydrant] Intersection of 21st Avenue and	J60486	9807	70	68	68.2	67.8	63.3	1.5	4.5	3.0	2.4	0.8	1.7
	581SE				11:22 AM	1	(Ditch)			1,449		85th Street Court				61			57.5	8.5	10.3	1.8			
47	581SE	SE Tacoma	5	10/13/2017	9:23 AM	1	Mid-block of Sweet Street, approximately 400 feet west of	J20364	7158	843	76	[West of Flow Hydrant] Mid-block of Sweet Street,	J20370	7155	69	66	74.3	69.1	66.7	3.0	2.4	-0.6	-0.3	1.7	-0.1
	581SE				9:24 AM	2	intersection of Sweet Street and Sonia Street			1,432		approximately 300 feet east of intersection of Sweet Street and [South of Flow Hydrant]				63			63.1	6.0	6.0	0.0			
48	581SE	SE Tacoma	5	10/13/2017	10:03 AM	1	Intersection of McDacer Avenue and North Lane	J22012	3691	908	82	On McDacer Avenue near intersection with 54th	J22018	3692	81	75	81.4	81.9	74.0	6.0	7.9	1.9	0.3	0.6	-0.9
	581SE				10:03 AM	1	(Catch Basin)			1,410		Street/Tanglewood Ave				63			65.2	18.0	16.8	-1.3			
49	581SE	SE Tacoma	5	10/13/2017	10:52 AM	1	D Street north of intersection with 77th Street	J19292	3749	924	79	[South of Flow Hydrant] D Street north of intersection with	J19290	3748	76	73	79.0	74.6	73.0	3.0	1.7	-1.3	-1.5	0.0	1.4
	581SE				10:54 AM	2	(Flow into woods to west) 8802 E Street (E Street Dead			1,669 893		78th Street [West of Flow Hydrant]				70			70.3 75.0	6.0	4.3	-1.7			
50	581SE 581SE	SE Tacoma	5	10/12/2017	1:07 PM 1:08 PM	2	End) (Catch Basin)	J16516	3795	1,582	79	Intersection of D Street and 88th Street (just east of intersection)	J16490	3752	78	74 72	77.2	77.2	75.0	4.0 6.5	2.2 5.5	-1.8 -1.0	-1.4	1.8	0.8
	581SE				12:08 PM	- 1	Intersection of D Street and S 67th			893		[North of Flow Hydrant]				76			74.8	3.0	5.2	2.2			
51	581SE	SE Tacoma	5	10/13/2017	12:09 PM	1	Street (Catch Basin)	J77518	3306	1,411	81	Intersection of D Street and 65th Street (just north of intersection, on D Street)	J77478	3305	79	70	80.8	80.0	68.2	9.0	11.8	2.2	2.5	0.2	-1.0
52	520SE	SE Tacoma	5	10/16/2017	1:42 PM	1	Intersection of 107th Street and 9th Avenue Court	J117868	9085	843	87	[North of Flow Hydrant] Towards 9th Avenue Court Dead	J116918	9083	80	55	86.4	78.0	51.4	24.9	26.6	1.7	2.0	0.6	1.9
52	520SE	OL TACUINA	5	10/10/2017	1:44 PM	2	(Catch Basin/Grass)	5117000	9000	1,388	07	End	0110010	3003	00	46	00.4	70.0	41.8	33.9	36.1	2.2	2.0	0.0	1.5
53	520SE	SE Tacoma	5	10/16/2017	1:59 PM	1	Intersection of 104th Street and 18th Avenue	J62610	9228	791	91	[East of Flow Hydrant] Intersection of 104th Street and	J62416	9245	87	65	91.1	87.3	64.5	22.0	22.9	0.9	0.2	-0.1	-0.3
	520SE		-		1:59 PM	2	(Grass/Vacant Lot)			1,261		Wilkeson Street (Just south of intersection on Wilkeson St)				61			61.7	26.0	25.6	-0.4			

### Tacoma Water Hydraulic Model Calibration Data 478 High Zone

								t Flow Teste	d			Hydrant for F	Pressure Mea	surements						Field Diff		i			
						Duration	Field M	F1 Model		Flow	Static	Field	R1 Model	nts		R1 Residual		R1 Static	R1 Residual	Pressure	Pressure	Error	Error	Static P Diff	Static P Diff
Test No.	Pr HGL (feet)		Figure No.	Date	Time	of Test (mins)	Location	Node No.	Hydrant No.	Calc (gpm)	Pressure (psi)	Location	Node No.	D/S Hydrant No.	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	F1 (psi)	F1 (psi)	(per flow) (psi)	(per site) (psi)	(Flow Hyd) (psi)	(Static Hyd) (psi)
54	478 478	- High	6	SKIPPED			On 94th Street, west of intersection with Ainsworth Ave (Catch Basin)	J17154	6464			[East of Flow Hydrant] On 94th Street, east of intersection with Ainsworth Ave	J76596	5631											
55	478	High	6	10/16/2017	9:47 AM	1	On 86th Street, east of intersection with J Street	J89956	2971	773	60	[West of Flow Hydrant] On 86th Street, approximately 7	J17568	2712	61	56	61.6	61.1	55.9	5.0	5.2	0.2	0.2	-1.6	-0.1
00	478	. iigii	Ű	10/10/2011	9:48 AM	2	(Catch Basin to west, at J Street)	000000	2071	1,259	00	houses west of intersection with J Street	011000	27.12	0.	50	01.0	0	50.1	11.0	11.1	0.1	0.2	1.0	0.1
56	478 478	High	6	10/16/2017	10:26 AM 10:27 AM	1	Intersection of 72nd Street and Fife Street (Catch Basin)	J18008	2058	939 1,563	89	[North of Flow Hydrant] Intersection of 72nd Street and Fife Street	J87566	2057	86	81 73	88.2	85.5	80.1 72.4	5.0 13.0	5.4 13.1	0.4 0.1	0.3	0.8	0.5
57	478	High	6	10/16/2017	11:06 AM	1	Intersection of 45th Street and Lawrence Street	J78158	1761	826	71	[South of Flow Hydrant] Intersection of 47th Street and	J24390	1762	66	61	71.0	64.9	60.9	5.0	4.0	-1.0	-1.0	0.0	1.1
	478				11:07 AM	1	(Catch Basin)			1,306		Lawrence Street				56			56.0	10.0	8.9	-1.1			
58	478 478	High	6	10/16/2017	11:28 AM 11:30 AM	1	Intersection of 58th Street and M Street (Catch Basin)	J22452	2698	716 1.039	54	[South of Flow Hydrant] Intersection of M Street and 59th Street	J22442	2699	51	47 45	53.3	49.8	47.5 45.8	4.0 6.0	2.3 4.1	-1.7 -2.0	-1.8	0.7	1.2
50	478				9:28 AM	1	Intersection of Cushman Ave and			675		[West of Flow Hydrant]				41			41.2	15.0	13.1	-1.9			
59 (Retest)	478	High	11	1/18/2018	9:29 AM	1	42nd Street (Catch Basin)	J25306	9796	983	53	Intersection of 42nd Street and Asotin Street	J25304	2483	56	27	53.0	54.3	28.2	29.0	26.2	-2.8	-2.3	0.0	1.7
60 (Retest)	478 478	High	11	1/18/2018	12:22 PM	1 	40th Street Ct cul-de-sac, west of intersection of 40th St Ct and East E Street	J26172	9355	631	59	[Southeast of Flow Hydrant] Intersection of East E Street and 43rd Street	J87784	3780	54	35	59.9	52.7	37.8	19.0	14.9	-4.1	-4.1	-0.9	1.3
61	478	High	11	1/18/2018	11:56 AM	1	(Catch Basin) Intersection of Wright Ave and G Street	J28986	3851	826	82	[West of Flow Hydrant] Intersection of Wright Ave and E	J28962	3774	80	60	81.9	79.5	62.2	20.0	17.3	-2.7	0.4	0.1	0.5
(Retest)	478	riigii		1/10/2010	11:57 AM	1	(Catch Basin)	320300	3651	1,259	02	[South of Flow Hydrant]	320302	3774	80	46	01.9	79.5	41.9	34.0	37.6	3.6	0.4	0.1	0.5
62	478 478	High	6	10/17/2017	10:09 AM 10:10 AM	1	Intersection of Cushman Ave and 23rd Street (Catch Basin)	J30570	2565	675 1,192	48	Cushman Ave and approx. 24th St (mid-block between 23rd and 25th)	J30754	2566	52	50 49	49.7	52.6	50.4 46.7	2.0 3.0	2.2 5.9	0.2 2.9	1.6	-1.7	-0.6
63	478 478	- High	8	10/17/2017	10:55 AM 10:55 AM	1	Intersection of 13th Street and Pine Street (Catch Basin)	J32184	1934	631 919	41	[East of Flow Hydrant] Intersection of 13th Street and Fife Street	J31966	2029	37	34 31	41.4	37.4	34.2 31.1	3.0 6.0	3.2 6.4	0.2 0.4	0.3	-0.4	-0.4
64	478 478	High	8	SKIPPED			Intersection of 8th Street and M Street (Catch Basin)	J8274	2718			[Northwest of Flow Hydrant] Intersection of 9th Street and M Street	J9074	2719											
65	478 478	High	8	10/17/2017	11:19 AM 11:20 AM	1	Intersection of 24th Street and Cedar Street (Catch Basin)	J9708	1885	924 1,328	69	[2 Blocks East of Flow Hydrant] Intersection of 24th Street and Rosemount (in elevated planter, under tree)	J9940	1975	75	71 62	68.4	73.8	68.4 63.3	4.0 13.0	5.4 10.5	1.4 -2.5	-0.5	0.6	1.2
66	478 478	High	8	10/17/2017	11:48 AM	1	Intersection of 36th Street and Monroe Street (Catch Basin)	J97398	1385	675 954	72	[2 Blocks East of Flow Hydrant] Intersection of 36th Street and Proctor Street	J40158	1461	74	71 67	72.9	71.1	67.1 63.8	3.0 7.0	4.0	1.0 0.3	0.7	-0.9	2.9
67	478	- High	8	10/17/2017	1:47 PM	1	Intersection of 42nd Street and Whitman Street	J84100	328	735	59	[East of Flow Hydrant] Intersection of 42nd Street and	J40666	361	64	60	57.6	63.1	58.9	4.0	4.2	0.2	1.6	1.4	0.9
L	478	Ť			1:49 PM	2	(Catch Basin)			1,328		Vassault Street				55			51.1	9.0	11.9	2.9			
68	478 478	High	8	10/17/2017	2:06 PM 2:06 PM	1 1	Approx. 2436 Ridgeview Drive (Catch Basin)	J69230	132	675 1,429	74	[Northeast of Flow Hydrant] Intersection of 27th Street and Diedra Circle	J38280	126	65	62 57	72.7	65.0	62.6 55.6	3.0 8.0	2.5 9.4	-0.5 1.4	0.4	1.3	0.0
69	478	High	8	10/18/2017	9:16 AM	1	710 Karl Johan Ave	J45156	42	1,067	115	[Northeast of Flow Hydrant] Fernside Drive and Terrace Drive	J106628	47	101	94	113.0	99.3	91.4	7.0	8.0	1.0	-0.1	2.0	1.7
	478				9:18 AM	2				1,769		Tomade Drive and Tenace Drive				80			79.4	21.0	19.9	-1.1			
70	478 478	High	8	10/18/2017	9:53 AM 9:54 AM	1	Aimsitram Church, on S. Baltimore Street	J45518	6759	809 1,430	69	[South of Flow Hydrant] S. Baltimore Street dead end	J45514	6760	63	60 58	67.3	61.8	59.6 55.6	3.0 5.0	2.2 6.1	-0.9	0.1	1.7	1.2
71	478	High	8	10/18/2017	10:24 AM	1	Leif Ericson Drive and Aurora Ave	J44708	58	716	70	[North of Flow Hydrant] Intersection of Aurora Ave and	J106386	57	72	67	69.2	72.3	68.6	5.0	3.7	-1.3	-2.6	0.8	-0.3
	478 478	riigii		10/10/2017	10:25 AM	1	Middle of Colgate Drive Loop,	344700	50	952	,,,	[Northwest of Flow Hydrant]	0100000	5,	12	60	00.2	72.0	64.1 92.4	12.0	8.2	-3.8	-2.0	0.0	-0.0
72	478 478	High	7	10/18/2017	10:47 AM 10:48 AM	1	South of 37th Street (Catch Basin to SW)	J109330	5755	984 1,634	95	[Northwest of Flow Hydrant] Intersection of Colgate Drive and 37th Street	J109348	6555	98	95 91	94.4	96.0	92.4 87.0	3.0 7.0	3.7 9.0	0.7 2.0	1.3	0.6	2.0
73	478	High	7	10/18/2017	11:21 AM	1	5218 97th Ave Ct	J3020	5782	1,040	104	[South of Flow Hydrant] Intersection of 97th Ave Ct and	J2714	5694	102	100	102.4	102.4	98.3	1.5	4.2	2.7	2.7	1.6	-0.9
	478				11:22 AM	2				1,767		54th Street Ct				94			92.1	7.5	10.3	2.8			

### Tacoma Water Hydraulic Model Calibration Data 478 High Zone

1			1	1	1	1	Hudrop	t Flow Teste	d			Hydrant for	Proceuro Mor	euromonte											1
								leasurements					Measureme					Model Resi	lte	Field Diff	Model Diff			Static P	Static P
						Duration		F1 Model	1	Flow	Static	1.04	R1 Model		R1 Static	R1 Residual	F1 Static		R1 Residual	Pressure	Pressure	Error	Error	Diff	Diff
Test		essure Zone				of Test		Node	Hydrant	Calc	Pressure		Node	D/S Hydrant	Pressure	Pressure	Pressure	Pressure	Pressure	F1	F1	(per flow)	(per site)	(Flow Hyd)	(Static Hyd)
No.	HGL (feet)	Descriptor	Figure No.	Date	Time	(mins)	Location	No.	No.	(gpm)	(psi)	Location	No.	No.	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)
74	478 478	High	7	SKIPPED			Approx. 6501 60th Street (Catch Basin)	J68226	8158			[500 feet East of Flow Hydrant] Approx. 6340 60th Street	J69902	8159											
75	478 478	High	7	10/19/2017	9:16 AM 9:17 AM	1 1	Intersection of 69th Street and Proctor Street (Catch Basin)	J93950	11657	998 1,719	102	[East of Flow Hydrant] 69th Street Dead End	J93956	11658	103	93 76	102.0	104.0	94.8 79.8	10.0 27.0	9.2 24.2	-0.8 -2.8	-1.8	0.0	-1.0
76	478 478	High	7	10/19/2017	9:43 AM 9:45 AM	1	Approx. 5400 54th Street (On 54th north of intersection with Cirque Drive) (Catch Basin)	J1106	9679	1,093 1,848	111	[East of Flow Hydrant] 54th Street Dead End	J1102	9680	99	94 80	111.1	98.2	90.0 77.3	5.0 19.0	8.2 20.9	3.2 1.9	2.6	-0.1	0.8
77	478 478	High	7	10/19/2017	10:14 AM 10:15 AM	1 1	Intersection of 29th Street and Adams Street (Catch Basin to west)	J100208	1497	716 1,306	51	[West of Flow Hydrant] 3852 29th Street (Entrance to Sunrise Ridge Apartments)	J43384	7306	53	53 50	51.6	52.5	51.3 49.2	0.0 3.0	1.2 3.3	1.2 0.3	0.8	-0.6	0.5
147	478 478	High	11	1/18/2018	8:54 AM 8:56 AM	1 2	Intersection of S. 45th Street and S. J Street (Catch Basin)	J25144	2863	653 1,066	40	[North of Flow Hydrant] Intersection of S. 43rd Street and S. J Street	J25148	2862	44	41 37	40.7	44.3	40.9 36.3	3.0 7.0	3.3 8.0	0.3 1.0	0.7	-0.7	-0.3
148	478 478	High	11	1/18/2018	9:50 AM 9:51 AM	1	Intersection of S. Ainsworth Ave and S. 36th Street (Catch Basin)	J25586	2529	695 1,092	54	[South of Flow Hydrant] Intersection of S. Ainsworth Ave and S. 37th Street	J25686	2528	55	54 54	54.2	53.7	53.2 52.8	1.0 1.0	0.5 0.9	-0.5 -0.1	-0.3	-0.1	1.3
149	478 478	High	11	1/18/2018	10:18 AM 10:19 AM	1 2	Intersection of S. 43rd Street and S. D Street (Catch Basin)	J26408	3291	695 1,092	56	[West of Flow Hydrant] Intersection of Fawcett Ave and S. 43rd Street	J78912	3256	53	53 50	55.9	51.6	50.5 49.4	0.5 3.0	1.1 2.2	0.6 -0.8	-0.1	0.1	1.4
150	478 478	High	11	1/18/2018	11:27 AM 11:28 AM	1 1	Intersection of E. 42nd Street E. N Street (Catch Basin)	J27312	11608	954 1,703	85	[Southwest of Flow Hydrant] Approx. 1329 E. 43rd Street.	J27300	11610	80	78 68	84.9	79.0	74.3 67.3	2.0 12.0	4.6 11.7	2.6 -0.3	1.2	0.1	1.0
151	478 478	High	11	1/18/2018	10:54 AM 10:55 AM	1 2	Intersection of E. 37th Street and E. K Street (Catch Basin)	J27400	4001	653 954	79	[North of Flow Hydrant] Intersection of E. 36th Street and E. K Street	J27402	4000	77	56 44	78.3	76.9	59.0 42.2	21.0 33.0	17.9 34.7	-3.1 1.7	-0.7	0.7	0.1
152	478 478	High	11	1/18/2018	2:21 PM 2:22 PM	1	Intersection of 37th Street and Spokane Street (Catch Basin)	J27468	3922	608 859	64	[North of Flow Hydrant] Intersection of E. 36th Street and Spokane Street	J27472	3921	64	46 36	63.6	62.1	45.8 32.6	18.0 28.0	16.3 29.5	-1.7 1.5	-0.1	0.4	1.9

### Tacoma Water Hydraulic Model Calibration Data West and NW Tacoma Operating Area (Not Including 478 High Zone)

Image: series         Image: series        Image: series        Image: se	<u> </u>							Hydran	Flow Tester	d			Hydrant for	Pressure Mea	surements											
							Duration	Field N	Easurements F1 Model		Flow	Static	Field	d Measuremer B1 Model	nts	B1 Static	R1 Residual	F1 Static	Model Results R1 Static	R1 Residual	Field Diff Pressure	Model Diff Pressure	Error	Error	Static P Diff	Static P Diff
1         1         2        <		P			-	-	of Test		Node		Calc	Pressure		Node		Pressure	Pressure	Pressure	Pressure	Pressure		F1	(per flow)	(per site)	(Flow Hyd)	(Static Hyd)
1     1	No.	HGL (feet)	Descriptor	Figure No.	Date	Time	(mins)		No.	No.	(gpm)	(psi)		No.	No.	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)		(psi)	(psi)	(psi)	(psi)
1100 <th0< td=""><td></td><td>556</td><td></td><td>_</td><td></td><td>12:30 PM</td><td>1</td><td></td><td></td><td></td><td>809</td><td></td><td></td><td></td><td></td><td></td><td>65</td><td></td><td></td><td>68.1</td><td>10.0</td><td>7.6</td><td>-2.4</td><td></td><td></td><td></td></th0<>		556		_		12:30 PM	1				809						65			68.1	10.0	7.6	-2.4			
1         2         2         2         1	78	EEC	Park Royal	7	10/18/2017	10-00 DM	0		J3178	5875	1 000	72		J2502	5876	75	<u>co</u>	73.9	75.8	57 C	15.0	10.1	0.1	0.4	-1.9	-0.8
<th< td=""><td></td><td>556</td><td></td><td></td><td></td><td>12.32 PW</td><td>2</td><td></td><td></td><td></td><td>1,309</td><td></td><td>Street</td><td></td><td></td><td></td><td>60</td><td></td><td></td><td>57.6</td><td>15.0</td><td>10.1</td><td>3.1</td><td></td><td></td><td></td></th<>		556				12.32 PW	2				1,309		Street				60			57.6	15.0	10.1	3.1			
3		556				1:00 PM	1				843						63			62.5	3.0	4.6	1.6			
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	79		Park Royal	7	10/18/2017		•		J1990	5904	1 000	73		J1764	5893	66		75.8	67.1	57.0		0.5	0.5	1.0	-2.8	-1.1
<th< td=""><td></td><td>556</td><td></td><td></td><td></td><td>T:UT PM</td><td>2</td><td></td><td></td><td></td><td>1,390</td><td></td><td>51st Street Ct</td><td></td><td></td><td></td><td>57</td><td></td><td></td><td>57.6</td><td>9.0</td><td>9.5</td><td>0.5</td><td></td><td></td><td></td></th<>		556				T:UT PM	2				1,390		51st Street Ct				57			57.6	9.0	9.5	0.5			
		531				12:00 PM	1				924		[Could of Elous Lindson)]				76			78.9	9.0	7.7	-1.3			
1         1 <th1< th=""> <th1< th=""> <th1< th=""></th1<></th1<></th1<>	80		University Place	7	10/19/2017				J73778	11759		88		J73772	11758	85		88.5	86.6					1.0	-0.5	-1.6
<table-container>           i<!--</td--><td></td><td>531</td><td></td><td></td><td></td><td>12:01 PM</td><td>1</td><td>(Catch Basin)</td><td></td><td></td><td>1,617</td><td></td><td>-</td><td></td><td></td><td></td><td>68</td><td></td><td></td><td>66.3</td><td>17.0</td><td>20.3</td><td>3.3</td><td></td><td></td><td></td></table-container>		531				12:01 PM	1	(Catch Basin)			1,617		-				68			66.3	17.0	20.3	3.3			
<table-container>  &lt;</table-container>		531				1:05 PM	1	East side of 53rd Street Ct dead			876						69			72.5	11.0	9.4	-1.6			
Image: bit image: bi	81		University Place	7	10/19/2017				J2308	8547		80		J2306	8546	80		82.7	81.9					-1.2	-2.7	-1.9
<		531				1:06 PM	1	(Catch Basin)			1,470						57			59.8	23.0	22.1	-0.9			
		531				1:51 PM	1	4510 Stand Augurus Ch			809						60			65.5	6.0	2.8	-3.2			
10         10        10         10         10	82		University Place	7	10/19/2017				J74538	591		67		J4984	5901	66		70.8	68.3					-2.2	-3.8	-2.3
<th< td=""><td></td><td>531</td><td></td><td></td><td></td><td>1:51 PM</td><td>1</td><td>(0000)</td><td></td><td></td><td>1,451</td><td></td><td></td><td></td><td></td><td></td><td>58</td><td></td><td></td><td>61.7</td><td>8.0</td><td>6.7</td><td>-1.3</td><td></td><td></td><td></td></th<>		531				1:51 PM	1	(0000)			1,451						58			61.7	8.0	6.7	-1.3			
i       i       i       issue and		290				1:33 PM	1				1,053		[East of Flow Hydrant]				84			88.8	19.0	14.8	-4.3			
i       i	83		Chambers Bay	7	10/18/2017				J3346	11029		127		J72132	11027	103		129.9	103.5					-0.3	-2.9	-0.5
4         5         0		290				1:34 PM	1				1,847						76			72.9	27.0	30.7	3.7			
i        i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <       i <     <		351				2:30 PM	1				675						41			42.2	9.0	5.8	-3.2			
1         1	84		Grandview	7	10/19/2017				J4154	5741		50		J75248	5743	50		47.7	48.0					-2.2	2.3	2.0
65       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70 <th< td=""><td></td><td>351</td><td></td><td></td><td></td><td>2:31 PM</td><td>1</td><td>(Gatch Basin)</td><td></td><td></td><td>1,143</td><td></td><td></td><td></td><td></td><td></td><td>40</td><td></td><td></td><td>39.2</td><td>10.0</td><td>8.8</td><td>-1.2</td><td></td><td></td><td></td></th<>		351				2:31 PM	1	(Gatch Basin)			1,143						40			39.2	10.0	8.8	-1.2			
65       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70       90       90       70 <th< td=""><td></td><td>202</td><td></td><td></td><td></td><td>9:25 AM</td><td>1</td><td>Intersection of F Day Island Blud</td><td></td><td></td><td>695</td><td></td><td></td><td></td><td></td><td></td><td>43</td><td></td><td></td><td>37.5</td><td>30.0</td><td>31.7</td><td>1.7</td><td></td><td></td><td></td></th<>		202				9:25 AM	1	Intersection of F Day Island Blud			695						43			37.5	30.0	31.7	1.7			
1         1	85	202	Day Island	7	10/20/2017	5.25 Aw			J118610	8373	000	80		J118538	8365	73		78.9	69.2	57.5	00.0	51.7	1.7	1.7	1.1	3.8
6       2d       1800       7       102007       303       2       688 Askee       3952       710       0       689 Askee       710      710      710     <		202	-					(Catch Basin - 1 PORT ONLY)					1916 Day Island Bivd W													
6       2d       1800       7       102007       303       2       688 Askee       3952       710       0       689 Askee       710      710      710     <		226				9:47 AM	1	Intersection of 19th Street and			860		[South of Flow Hydrant]				62			63.0	10.0	6.4	-3.6			
1       1 <th1< th=""> <th1< th=""> <th1< th="">    &lt;</th1<></th1<></th1<>	86	220	Titlow	7	10/20/2017	5.47 AW			J93582	7407	000	74		J44108	7408	72	02	72.6	69.5	03.0	10.0	0.4	-3.0	-2.6	1.4	2.6
1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 + 1 +		226				9:50 AM	2	(Catch Basin)			1,526						55			54.1	17.0	15.3	-1.7			
1       1		200		1		10.00 414		On 27th Street, just east of			770		[East of Flow Hydrapt]				54			50.0	4.0	5.0	1.0			
1       3       1 <th1< th=""> <th1< th=""> <th1< th=""></th1<></th1<></th1<>	87	320	Narrows	7	10/20/2017	10.20 AIVI			J6960	5843	113	57		J44072	7628	58	34	57.3	55.2	50.0	4.0	5.3	1.3	2.0	-0.3	2.8
18       33       Narose       7       102007       1116 AM       1       Approx 91 Mounds Wer Ave Wer Liver MU and 100 Sinet       0000       774       80       00       Pictor Flow Hydenily Mu and 100 Sinet       100000       775       70       4.1       2.9       2.8       2.9       0.4         9       581 F       Flocher Heights       8       1020001       10 PM       2       10000000       10000000       100		328				10:22 AM	2				1,326						48			42.6	10.0	12.7	2.7			
8       Numme       7       102007       1118 M       2       With each or street       106 mode       106 mode       87       74       80       817       817      <		000						(**************************************			000		(Nexth of Flow Liverant)							70.7	7.0					
1       1	88	328	Narrows	7	10/20/2017	11:16 AM	1		J45070	7744	860	80		J106536	7745	87	80	80.4	83.7	/9./	7.0	4.1	-2.9	-2.9	-0.4	3.3
9       307       102 (2017)       1.0 PM       2.0 solution of indicasenetic on with the second of indicas		328				11:18 AM	2				1,563						74			73.5	13.0	10.2	-2.8			
9       307       102 (2017)       1.0 PM       2.0 solution of indicasenetic on with the second of indicas								On Washington Street, ~300 feet																		
$ \begin{array}{                                     $	89	581F	Eletcher Heights	8	10/20/2017	1:08 PM	1	south of intersersection with 19th	.143952	9342	939	82		.193422	6227	87	84	85.8	87.3	84.5	3.0	2.8	-0.2	-0.2	-3.8	-0.3
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	00	581F	r lotonor riolgino		10/20/2011	1:10 PM	2		0.0002	0012	1,686	02		000122	OLL!	0,	79	00.0	07.0	79.6	8.0	7.7	-0.3	0.2	0.0	0.0
9       5ex       Fetcher Height       8       1020201       149 Pt       2       15th Steet       4620       1100       72       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       80.1       75       64       66.1       66.9      66.9       66.9																										
587       0       0       148 PM       2       (Catch Bain)       1546       14h Stret       0       64       0       62       10       127       17       <	00	581F	Elotobor Hoighto		10/20/2017	1:47 PM	1		146200	11000	893	76		146064	1201	75	69	90.1	75.5	70.7	6.0	4.9	-1.1	0.2	26	-0.5
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	50	581F	r letcher neights		10/20/2017	1:49 PM	2		340200	11002	1.546	70		340204	1301	75	64	00.1	75.5	62.9	11.0	12.7	1.7	0.5	-3.0	-0.5
91 $\overline{10232017}$ $\overline{10232017}$ $\overline{001}$ M         2         Washington Street (Cather Basin) $\overline{10232017}$ <								, ,																		
1       538 <sup>2</sup>	01	538F	Flotobor		10/23/2017	8:59 AM	1		183694	9057	716	66		146000	1565	64	56	66.0	65.2	58.3	8.5	7.0	-1.5	-16	-0.0	-1.3
$ \begin{array}{c} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$	51	538F	rielGrier	0	10/23/2017	9:01 AM	2		000004	3331	1,262	00		040000	1000	04	47	00.9	03.3	49.5	17.5	15.8	-1.7	-1.0	-0.9	-1.5
92       53F       Fletcher       8       10232017       923AM       2       Mulein Street (Catch Basin)       1306       71       Intersection of Bith Street and Husson Street       J4650       1028       73       60       72.9       76.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       60.0       71.0       70.0      <								( ,																		
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	00	538F	Floteber		10/02/0017	9:21 AM	1		100540	1101	809	71		146500	1000	70	64	70.0	76.0	68.2	9.0	7.8	-1.2	0.0	1.0	-3.0
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	92	538F	FIEIGHER	0	10/23/2017	9:23 AM	2		J0JJ40	1101	1.306	/1		J4030U	1020	73	60	72.9	76.0	60.2	13.0	15.7	2.7	0.0	-1.9	-3.0
93       53W       Westgate       8       10232017       944 AM       2       Cheyenes Streted Bin       1281       70       Intersection of Stevens Streted 18th Streted       1680 300       1233       64       -       71.1       65.9       57.8       9.0       8.2       0.0       1.1       1.2 </td <td></td>																										
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	02	538W	Wostaata		10/02/0017	9:42 AM	1		160000	1144	826	70		160000	1000	64	60	71.1	SE O	61.1	4.0	4.9	0.9	0.0	1.1	-1.9
$ \begin{array}{c} \\ \hline 94 \\ \hline 538 \\ \hline 538 \\ \hline 538 \\ \hline 538 \\ \hline \\ 538 \\ \hline \\ \hline \\ \\ 538 \\ \hline \\ \hline \\ \\ \hline \\ 538 \\ \hline \\ \hline \\ \\ \hline \\ \\ 538 \\ \hline \\ \hline \\ \\ \hline \\ \\ 538 \\ \hline \\ \hline \\ \\ \hline \\ \\ \\ \\ \\ \hline \\ \\ \\ \\ \\ \\ \hline \\$	უკ	538W	vv estgate	ð	10/23/2017	9:44 AM	2		109835	1141	1,261	70		109808	1233	64	55	71.1	65.9	57.8	9,0	8,2	-0.8	0.0	-1.1	-1.9
$ \begin{array}{c} 94 \\ \hline 96 \\ \hline 96 \\ \hline 96 \\ \hline 97 \\ \hline 96 \\ \hline 97 \\ \hline 97 \\ \hline 96 \\ \hline 9$								· · · ·																		
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$		538W	Westernt-		10/00/0017	10:08 AM	1		107000	2022	773	60		100001	015	61	56	c0 c	co 1	55.3	5.0	6.8	1.8	0.5	0.0	
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $	94	538W	vv estgatë	ð	10/23/2017	10:10 AM	2		J37006	293	1,306	60		J68964	315	61	50	60.6	62.1	48.1	11.0	14.1	3.1	2.5	-0.6	-1.1
$ \begin{array}{c} 95 \\ \hline 96 \\ \hline 9$							2	childhoo to Woolgato arcon																		
1       538W       0       1       1053 AM       2       (Catch Basin)       1216       intersection with 30th Street)       0       43       0       41.3       13.0       16.4       3.4       0       1         36W       446       North End       8       1024/2017       11.27 AM       1       Intersection of 36th Street and Gove Street (Catch Basin)       477       41       [[North of Flow Hydrant]]       14000       17.7       44       [[North of Flow Hydrant]]       14000       17.7       44       41.7       49.2       47.0       48.0       4.4       3.6       2.2       2.8       3.2       2.0       2.2       2.8       3.3       2.2       2.2       2.8       3.3       2.2       2.2       2.8       3.3       2.2       2.2       2.8       3.3       2.2       2.3       3.3       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.4       3.3       3.3       3.4       3.3 <td>05</td> <td>538W</td> <td></td> <td></td> <td>10/00/2017</td> <td>10:52 AM</td> <td>1</td> <td></td> <td>1000770</td> <td></td> <td>716</td> <td></td> <td></td> <td>174 000</td> <td>007</td> <td></td> <td>51</td> <td></td> <td></td> <td>50.3</td> <td>5.0</td> <td>7.4</td> <td>2.4</td> <td></td> <td></td> <td></td>	05	538W			10/00/2017	10:52 AM	1		1000770		716			174 000	007		51			50.3	5.0	7.4	2.4			
446       North End       8       1024/2017       11:27 AM       1       Intersection of Baltimeter (Catch Basin)       477       41       [[North of Flow Hydran1]]       14:000       11:74       44       44.1       49.2       47.0       5.0       2.2       -2.8       -3.2       -0.7       -3.2       -0.7       -3.2	95	538W	Westgate	8	10/23/2017	10:53 AM	2		J38872	443	1 216	55		J71388	385	56	43	56.8	57.6	41.3	13.0	16.4	3.4	2.9	-1.8	-1.6
96       46       North End       8       10/24/2017       11:28 AM       2       Gove Street (Catch Basimore Street and Ruby Street       41       Intersection of Gove Street and 38th Street       J40006       1174       49       41.7       49.2       44.8       8.0       4.4       -3.6       -3.2       -0.7		330W				10.55 AIVI	2				1,210		,				40			41.5	13.0	10.4	0.4			
446		446				11:27 AM	1				477						44			47.0	5.0	2.2	-2.8			
97       446       North End       8       10/24/2017       11:55 AM       1       Intersection of Batimore Street and Ruby Street       91       1,026       96       [West of Flow Hydraril] Intersection of Ruby Street and J105206       856       101       95       94.1       100.6       93.5       6.0       7.2       1.2       0.0       1.9	96	446	North End	8	10/24/2017	11-29 414	<u>م</u>		J39916	1173	719	41		J40006	1174	49	41	41.7	49.2	44.9	0.0	4.4	26	-3.2	-0.7	-0.2
97 North End 8 10/24/2017 and Ruby Street J105232 911 96 Intersection of Ruby Street and J105206 856 101 94.1 100.6 0.0 1.9		440				11.20 AM	2	(Gatch Dasin)			/10		Sour Sueer				41			44.0	0.0	4.4	-3.0			
		446				11:55 AM	1				1,026						95			93.5	6.0	7.2	1.2			
440 11.37 AWI 2 (Catch Basin) 1,018 Shiney Street 84 84.8 17.0 15.8 -1.2	97	440	North End	8	10/24/2017	11.57 414	0		J105232	911	1.010	96		J105206	856	101	04	94.1	100.6	04.0	17.0	15.0	1.0	0.0	1.9	0.4
		446		1		11:57 AM	2	(Catch Basin)			1,618		Sniney Street				84			84.8	17.0	15.8	-1.2			

Tacoma Water
Hydraulic Model Calibration Data
West and NW Tacoma Operating Area (Not Including 478 High Zone)

-							Hydran	Flow Tested	1			Hydrant for F	Pressure Mea	surements											
Test No.	Pr HGL (feet)	essure Zone Descriptor	Figure No.	Date	Time	Duration of Test (mins)	Field M Location	easurements F1 Model Node No.	Hydrant No.	Flow Calc (gpm)	Static Pressure (psi)	Location	d Measuremer R1 Model Node No.	Hydrant No.	R1 Static Pressure (psi)	R1 Residual Pressure (psi)	F1 Static Pressure (psi)	Model Results R1 Static Pressure (psi)	R1 Residual Pressure (psi)	Field Diff Pressure F1 (psi)	Model Diff Pressure F1 (psi)	Error (per flow) (psi)	Error (per site) (psi)	Static P Diff (Flow Hyd) (psi)	Static P Diff (Static Hyd) (psi)
98	446 446	North End	8	10/24/2017	12:23 PM 12:25 PM	1 2	Intersection of 50th Street and Frace Avenue (Catch Basin)	J41064	290	939 1,508	80	[North of Flow Hydrant] Intersection of 51st Street and Frace Avenue	J41056	291	96	93 85	80.2	95.6	91.2 86.1	3.0 11.0	4.4 9.5	1.4 -1.5	0.0	-0.2	0.4
99	350 350	Salmon Beach	8	10/20/2017	12:39 PM 		NO FLOW - RECORD PRESSURE ONLY	J41268	11613			Obtain pressure measurement on first hydrant on Salmon Beach Road (east side of road, approx. 500 feet north of 51st Street)	J41268	11613	75			74.9							0.1
100	346 346	Old Town	8	10/23/2017	1:16 PM 	1	Intersection of Alder Street and 32nd Street (Catch Basin - 1 PORT ONLY)	J10666	7430	695	64	[East of Flow Hydrant] Intersection of Cedar Street and 32nd Street	J10698	1889	67	49 	63.7	66.8	52.0	18.0	14.8	-3.2	-3.2	0.3	0.2
101	446 446	Middle	8	10/24/2017	10:05 AM 10:06 AM	1 2	Intersection of 6th Street and C Street (Catch Basin - Use sandbags to contain water in catch basin	J79426	3421	969 1,635	103	[West of Flow Hydrant] Intersection of 7th Street and C Street	J79422	3422	112	100 83	101.3	112.1	99.1 79.4	12.0 28.6	13.0 32.7	1.0 4.1	2.5	1.7	-0.1
102	446 446	Middle	6	10/24/2017	8:59 AM 9:01 AM	1 2	Approx. 2335 South G Street (west side of street)	J67310	3093	843 1,507	80	[South of Flow Hydrant] Intersection of South G Street and Court G	J67306	11815	81	75 67	78.5	80.4	75.6 67.9	6.0 14.0	4.9 12.6	-1.1 -1.4	-1.3	1.5	0.6
103	446 446	Middle	8	10/24/2017	9:29 AM 9:31 AM	1 2	Intersection of 16th Street and South G Street (Catch Basin)	J33004	3087	843 1,369	70	[North of Flow Hydrant] Intersection of 15th Street and South G Street	J33104	3086	71	66 64	70.1	70.6	67.8 64.9	5.0 7.0	2.8 5.6	-2.2 -1.4	-1.8	-0.1	0.4
145	346 346	Portland Ave.	6	11/1/2017	9:18 AM 	1	Intersection of E 34th Street and E. R Street (Catch Basin)	J28120	4197	969	120	[North of Flow Hydrant] Intersection of E. R Street and E. Wright Street	J28062	11606	122	90	120.0	121.7	86.3	32.0	35.4	3.4	3.4	0.0	0.3
146	346 346	Portland Ave.	6	11/1/2017	9:55 AM 9:57 AM	1 2	Intersection of Harrison Street and E. T Street	J28104	4220	893 1,216	110	[East of Flow Hydrant] Intersection of E. Roosevelt Ave and E Harrison Street	J27934	4265	73	46 24	110.5	73.0	43.2 27.5	27.0 49.0	29.9 45.6	2.9 -3.4	-0.3	-0.5	0.0

### Tacoma Water Hydraulic Model Calibration Data Port of Tacoma and Fife Heights Operating Area

-	<u>г</u>		1 1		1		Hvdran	t Flow Tester	1			Hydrant for	Pressure Mea	surements		1									
							Field M	leasurements				Field	d Measuremen	ts				Model Results			Model Diff			Static P	Static P
		-				Duration		F1 Model		Flow	Static		R1 Model			R1 Residual	F1 Static	R1 Static	R1 Residual	Pressure	Pressure	Error	Error	Diff	Diff
Test No.	HGL (feet)	ssure Zone Descriptor	Figure No.	Date	Time	of Test (mins)	Location	Node No.	Hydrant No.	Calc (gpm)	Pressure (psi)	Location	Node No.	Hydrant No.	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	F1 (psi)	F1 (psi)	(per flow) (psi)	(per site) (psi)	(Flow Hyd) (psi)	(Static Hyd) (psi)
	251				8:45 AM	1	Intersection of 29th Street and			826		[East of Flow Hydrant]			4 - 7	74	W - 7		71.7	5.0	5.6	0.6	u - 7	4-7	w - 7
104	251	Low	8	10/25/2017	8:47 AM	2	Carr Street (Catch Basin)	J72200	6640	1,542	81	Intersection of 29th Street and McCarver Street	J72204	3210	79	71	79.6	77.3	71.1	8.0	6.2	-1.8	-0.6	1.4	1.7
105	251	Low	9	10/25/2017	9:54 AM	1	Approx. 1160 Dock Street (2nd hydrant north on Dock Street	J34926	3560	1,053	100	[North of Flow Hydrant]	J34934	9850	102	100	100.4	100.2	99.0	2.0	1.2	-0.8	-0.3	-0.4	1.8
100	251	2011	Ŭ	10/20/2017	9:56 AM	2	from intersection with E 15th Street - Catch Basin)	001020	0000	1,952		Approx. 1140 Dock Street	00.001	0000	.02	99	100.1	100.2	97.1	3.0	3.1	0.1	0.0	0.1	
106	251	Low	9	10/25/2017	11:40 AM	1	East side of 30th Street dead end, east of intersection with R Street	J28804	6228	1,012	94	[West of Flow Hydrant] Intersection of 30th Street and R	J28806	4187	91	89	94.3	89.3	87.8	2.0	1.6	-0.4	-0.5	-0.3	1.7
	251	-			11:41 AM	2	(Grass/vacant lot)			1,753		Street		-		86			85.0	5.0	4.3	-0.7			
107	251	Low	9	10/25/2017	10:22 AM	1	Approx. 750 feet east of intersection of E. D Street and E.	J34466	5554	1,053	101	[West of Flow Hydrant] Approx. 350 feet east of	J86928	3768	101	99	100.0	99.8	96.9	2.0	2.9	0.9	1.2	1.0	1.2
	251				10:24 AM	2	18th Street. (Next to Supervalu warehouse)			1,778		intersection of E. D Street and E. 18th Street.				95			92.4	6.0	7.5	1.5			
108	251	Low	9	10/25/2017	11:07 AM	1	On 26th Street approx. 400 feet	J101884	3927	1,012	103	[West of Flow Hydrant] On 26th Street approx. 400 feet	J101850	3872	102	96	101.9	101.8	94.2	6.0	7.6	1.6	2.4	1.1	0.2
	251	-			11:09 AM	2	west of intersection with J Street			1,703		east of intersection with G Street				85			81.6	17.0	20.2	3.2			
109	251	Low	9	10/25/2017	12:49 PM	1	Approx. 1380 Thorne Road	J79438	4387	1,053	99	[Northwest of Flow Hydrant] Approx, 375 feet northwest of flow	J7602	4386	99	98	98.3	98.1	96.6	1.0	1.5	0.5	0.5	0.7	0.9
	251	-			12:51 PM	3	(Catch Basin)			1,862		hydrant on Thorne Road				96			94.7	3.0	3.4	0.4			
110	251	Low	9	10/25/2017	1:30 PM	1	Approx. 120 Alexander Ave (On west side of road, next to	J11020	4434	1,040	100	[Northwest of Flow Hydrant] On Alexander Ave, across from	J74220	4433	99	94	98.4	98.0	89.8	5.0	8.2	3.2	1.4	1.6	1.0
	251		÷		1:32 PM	2	catch basin)			1,768		TOTE driveway entrance (on east side of road)				78			77.3	21.0	20.7	-0.3			
111	251	Low	9	10/25/2017	1:56 PM	1	On Marine View Drive, just south of home with address 4532 Marine	J70872	4556	1,026	95	[North of Flow Hydrant] On Marine View Drive, just north	J70874	4557	97	91	95.7	95.2	85.1	6.0	10.1	4.1	3.3	-0.7	1.8
	251				1:58 PM	2	View Drive (Flow to west in grass towards		.200	1,720	20	of home with address 4532 Marine View Drive			2.	74		10.2	69.7	23.0	25.5	2.5	2.0	5.7	
112	411	Fife Heights	9	10/24/2017	1:45 PM	1	Approx. 10 66th Avenue (Just south of intersection with 1st	J52378	8053	608	81	[North of Flow Hydrant] On 66th Avenue, just north of	J51764	4827	58	48	51.5	56.5	48.1	10.0	8.4	-1.6	-1.6	29.5	1.5
	411		Ĵ	10/2 1/2017			Street) (Ditch - 1 PORT ONLY)	002070	0000		51	intersection with 1st Street	00.704	.527	50		01.0	00.0						20.0	
113	411	Fife Heights	9	10/24/2017	2:28 PM	1	Intersection of 11th Street and 67th Ave Ct	J52326	7398	1,053	111	[West of Flow Hydrant] Intersection of 11th Street and	J92792	7399	104	93	111.2	105.5	92.9	11.0	12.6	1.6	1.6	-0.2	-1.5
	411	rioignito	Ŭ				(Catch Basin - Likely Only 1 Port)	-02020				67th Ave Ctz	302.02											0.2	

### Tacoma Water Hydraulic Model Calibration Data 549 NE Tacoma Zone

						Hydrant Flow Tested					Hydrant for Pressure Measurements													
					Field N	feasurements F1 Model		Flow	Static	Field Meas	surements Model		R1 Static	D1 Decidu	Model Results F1 Static R1 Static R1 Re		R1 Residual	Field Diff Pressure	Model Diff Pressure	Error	Error	Static P Diff	Static P Diff	
Test	Pressure Zone				Duration of Test		Node	Hydrant	Calc	Pressure			Hydrant	Pressure	Pressure	Pressure	Pressure	Pressure	F1 F1	F1	(per flow)	(per site)	(Flow Hyd)	
No.	HGL (feet) Descriptor	Figure No.	Date	Time	(mins)	Location	No.	No.	(gpm)	(psi)		No.	No.	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(per site) (psi)	(now nyu) (psi)	(psi)
114	549 	ia 9	10/26/2017	11:14 AM	1	Approx. 6000 6th Street (Catch Basin/Ditches)	J96292	9633	653	59	[West of Flow Hydrant] Intersection of 6th Street and 59th Ave Ct	09118	4842	55	40	60.0	56.1	42.0	15.0	14.1	-0.9	-0.9	-1.0	-1.1
	549			11:45 AM	1				809		[East of Flow Hydrant]				58			57.2	10.0	10.8	0.8			
115	549 NE Tacon	ia 9	10/26/2017	11:47 AM	2	Intersection of 56th Ave and 17th Street Ct	J51920	7101	1.348	64		6354	7102	68	43	65.3	68.1	43.4	25.0	24.7	-0.3	0.3	-1.3	-0.1
	549			12:47 PM	-	South side of 63rd Ave, just south			826						58			59.5	11.0	11.1	0.1			
116	549 NE Tacon	ia 9	10/26/2017	12:49 PM	2	of intersection with 23rd Street (Catch Basin)	J11614	7984	1.284	71	[Northwest of Flow Hydrant] Approx. 6225 23rd Street J1	1606	8054	69	44	72.1	70.6	47.7	25.0	22.9	-2.1	-1.0	-1.6	-1.6
	549			1:37 PM	1	Intersection of 27th Street and			908		[South of Flow Hydrant]				70			69.2	2.0	3.6	1.6			
117	NE Tacon 549	ia 10	10/26/2017	1:40 PM	2	59th Ave (Catch Basin)	J86676	4804	1,580	74	Intersection of 59th Avenue and J1 26th Street	1820	4817	72	66	75.3	72.7	64.9	6.0	7.8	1.8	1.7	-1.3	-0.7
	549			10:10 AM	1	Intersection of SW 341st Street			876		[West of Flow Hydrant]				70			70.4	2.0	3.2	1.2			
118	549 NE Tacon	ia 10	10/27/2017	10:12 AM	2	and 32nd Ave SW (Catch Basin)	J64476	4869	1,582	66	Approx. 34025 SW 33rd Ave	4460	9537	72	66	69.2	73.6	66.7	6.0	6.9	0.9	1.0	-3.2	-1.6
119	549 NE Tacon	ia 10	10/27/2017	10:46 AM	1	Intersection of Spyglass Drive and 36th Street NE	J74574	7625	1,067	101	[North of Flow Hydrant]	4672	7622	101	98	100.1	100.4	98.2	3.0	4.2	1.2	15	-1.1	-1.4
119	549	ia iu	10/27/2017	10:47 AM	2	(Catch Basin)	J/45/4	7625	1,827	101	Approx. 3630 Spyglass Drive	4072	/022	101	94	102.1	102.4	93.7	7.0	8.8	1.8	1.5	-1.1	-1.4
120	549 NE Tacon	ia 10	10/27/2017	11:23 AM	1	Intersection of Larchmont Ave NE and Braeburn Drive NE	J13730	4589	716	70	[North of Flow Hydrant] Intersection of Larchmont Ave NE J13	3818	4590	73	70	71.4	72.2	68.9	3.0	3.3	0.3	0.1	-1.4	0.8
120	549	ia iu	10/27/2017	11:24 AM	1	(Catch Basin)	313730	4389	1,164	70	and Oakmont Street NE	3010	4390	73	66	71.4	12.2	65.4	7.0	6.8	-0.2	0.1	-1.4	0.8
121	549 NE Tacon	ia 10	10/27/2017	12:51 PM	1	Approx. 3628 SW 331st Place	J83018	4890	860	70	[East of Flow Hydrant] Intersection of SW 331st Place J8	3016	4889	77	74	68.6	76.6	74.0	3.0	2.6	-0.5	-0.5	1.4	0.4
	549			12:52 PM	2	(Catch Basin)			1,563		and 36th Avenue SW				71			71.0	6.0	5.5	-0.5			
122	549 NE Tacon	ia 10	10/30/2017	9:10 AM	1	Intersection of 47th Street NE and 29th Avenue NE	J74306	7224	695	48	[South of Flow Hydrant]	4236	7225	49	47	47.8	48.6	46.4	2.0	2.3	0.3	0.6	0.2	0.4
	549			9:11 AM	2	(Catch Basin)			1,208		29th Ave NE cul-de-sac		-		44			42.6	5.0	6.0	1.0			
123	549 NE Tacon	ia 10	10/30/2017	9:31 AM	1	Approx. 5518 Green Hills Ave NE (Catch Basin)	J87156	4658	791	62	[Northeast of Flow Hydrant] Intersection of Browns Point Blvd NE and 57th Street NE - Same	4748	4689	56	55	61.2	53.6	51.3	1.0	2.3	1.3	2.4	0.8	2.4
	549	_		9:32 AM	2	(Gatori Basiri)			1,410		hydrant as NE13				54			47.9	2.0	5.6	3.6			
124	549 NE Tacon	ia 10	10/30/2017	9:46 AM	1	Approx. 1863 Overview Drive NE (Catch Basin)	J15118	4693	954	83	[East of Flow Hydrant] Approx, 1891 Overview Drive	2318	4694	87	85	81.3	85.4	82.5	2.0	2.9	0.9	1.8	1.7	1.7
	549			9:48 AM	2	(Gaton Basin)			1,719		Approx. 1691 Overview Drive				82			77.6	5.0	7.7	2.7			

#### Tacoma Water Hydraulic Model Calibration Data NE Tacoma Operating Area (Not Including 549 NE Tacoma Zone)

								t Flow Teste	d			Hydrant for F									Chatia D				
						Duration	Field N	F1 Model		Flow	Static	Field	d Measuremer R1 Model	nts	R1 Static	R1 Residual	F1 Static	R1 Static	R1 Residual	Field Diff Pressure	Model Diff Pressure	Error	Error	Static P Diff	Static P Diff
Test No.	Pre HGL (feet)	essure Zone Descriptor	Figure No.	Date	Time	of Test (mins)	Location	Node No.	Hydrant No.	Calc (gpm)	Pressure (psi)	Location	Node No.	Hydrant No.	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	Pressure (psi)	F1 (psi)	F1 (psi)	(per flow) (psi)	(per site) (psi)	(Flow Hyd) (psi)	(Static Hyd) (psi)
	370				10:25 AM	1	Intersection of Pointe Woodworth			954	Q - 7					76	u - 7		72.5	4.5	6.0	1.5		a - 7	
125		Overlook	10	11/2/2017		-	Dr and Beacon Ct	J76336	8132		81	[Northeast of Flow Hydrant] Beacon Ct cul-de-sac	J76298	8133	81		79.5	78.5					2.3	1.6	2.0
	370				10:27 AM	2	(Catch Basin)			1,669						70			65.3	10.1	13.2	3.1			<b></b>
126	486	Devertu Lleinhte	10	11/2/2017	9:47 AM	1	Approx. 2937 37th Ave	J11278	7360	924	87	[North of Flow Hydrant]	J11274	7361	89	75	87.7	88.1	78.1	14.0	9.9	-4.1	-1.7	-0.7	1.0
120	486	Beverly Heights	10	11/2/2017	9:49 AM	2	(Catch Basin)	J112/0	7360	1,527	0/	37th Ave near cul-de-sac	J112/4	/301	09	68	07.7	00.1	66.3	21.0	21.8	0.8	-1.7	-0.7	1.0
	649				10:52 AM	1				924		[Southeast of Flow Hyrdrant]				80			81.9	6.0	4.8	-1.2			
127		Indian Hill	10	10/30/2017			Approx. 3517 46th Street NE (Catch Basin to East)	J14012	7818		72	Intersection of 36th Ave NE and	J73004	7817	86		71.3	86.6					1.1	0.8	-0.6
	649				10:54 AM	2	· · · · · ·			1,430		45th Street NE				72			69.2	14.0	17.4	3.4			
	649				11:18 AM	1	Approx. 3420 Laurelwood Circle NE			826		[Northwest of Flow Hydrant] Approx. 3460 Laurelwood Circle				72			71.8	6.0	4.0	-2.0			1
128	649	Indian Hill	10	10/30/2017	11:20 AM	2	(SE side of Laurelwood Circle)	J15410	8167	1,350	72	NE	J15394	8168	78	59	70.0	75.8	59.7	19.0	16.1	-2.9	-2.5	2.0	2.2
						2	(Catch Basin)					(NW side of Laurelwood Circle)										-			<b></b>
129	649	Indian Hill	10	10/30/2017	11:44 AM	1	Intersection of Ridge Dr NE and Davis Ct NF	J15898	7532	809	66	[East of Flow Hydrant]	J15896	7533	62	53	66.7	61.1	53.9	9.0	7.2	-1.8	-0.8	-0.7	0.9
	649	indian min		10/00/2011	11:46 AM	2	(Catch Basin)	010000	1002	1,306	00	Davis Ct NE cul-de-sac	010000	1000	02	40	00.1	0	38.8	22.0	22.3	0.3	0.0	0.7	0.0
	426				8:55 AM							On Slayden Road, between													
130		Harbor View	10	11/2/2017								intersections with Varco Road and	J87154	4623	64			64.3							-0.3
	426											Kennedy Road													
131 -	346	Browns and			1:18 PM		Intersection of Marine View Drive					[Northeast of Flow Hydrant]													
Static s	346	Dash Point	10	10/30/2017			and Heron Ridge Drive (Catch Basin - 1 PORT ONLY)	J14514	8118		72	Heron Ridge Drive dead end	J76690	8120	36		71.4	34.9						0.6	1.1
					2:44 PM		,			000		End of Elevel balance)				10			50.7						
131 -	346	Browns and	10	10/30/2017	2:44 PM	1	Approx. 1040 51st Street NE In landscaped area near 5	J14824	9571	809	70	[East of Flow Hydrant] Just north of intersection of 51st	J14848	7322	59	48	68.3	59.6	50.7	11.0	8.9	-2.1	-2.1	1.7	-0.6
Rev.	346	Dash Point					mailboxes					Street and Caledonia Road NE													1
	346	Drawna and			1:39 PM	1	In grass near intersection of East			809		[Southeast of Flow Hydrant]				34			35.0	5.0	3.3	-1.7			
132	040	Browns and Dash Point	10	10/30/2017	1:40 PM	0	Side Drive NE and Whittier Street	J101386	4716	1 201	70	Intersection of Whittier Street NE and Dash Point Blvd NE	J59774	4936	39	01	66.9	38.2	01.0	0.0	6.4	1.0	-1.7	3.1	0.8
	346				1:40 PM	2	(Flow into grassy area) Approx. 6900 Water Street NE			1,391		[North of Flow Hydrant]				31			31.8	8.0	6.4	-1.6			<u> </u>
133	346	Browns and	10	10/30/2017	2:00 PM	1	(Catch Basin - 1 PORT ONLY -	J59626	7672	1,067	111	On east side of Water Street NE,	J59624	7674	118	108	109.7	118.4	106.7	10.0	11.7	1.7	1.7	1.3	-0.4
133	346	Dash Point	10	10/30/2017			Use sandbags to contain flow in downstream catch basin)	333020	1012			just south of fork in road with Alder Street NF sign	333024	7074	110		103.7	110.4					1.7	1.5	-0.4
	226				11:11 AM	1	Just north of intersection of Wa			735						47			48.3	10.9	7.6	-3.3			
134	-	Hayada	10	11/2/2017			Tau Ga Ave NE and Mana Wana	J14838	7916		54	[North of Flow Hydrant] Wa Tau Ga Ave NE dead end	J14886	7915	58		53.3	55.9					-2.3	1.1	2.0
	226				11:13 AM	2	Place NE			1,261						45			44.3	12.9	11.6	-1.3			
	411				11:53 AM	1	On 21st Ave NE at intersection			735		[North of Flow Hydrant]				61			65.7	8.0	4.4	-3.6			
135	411	Dash Point High	10	11/2/2017	11:54 AM	2	(Catch Basin)	J100722	7164	1,328	59	On 21st Ave at intersection with Austin Road NE	J75302	4732	69	58	59.0	70.1	60.3	11.0	9.7	-1.3	-2.4	0.0	-1.1
	000				10-41 DM		Intersection of Soundview Drive			75.4		[Couth of Flow   Judroot]				60			CD 4	22.0	01.0	-0.4			
136	226	Dash Point Low	10	11/2/2017	12:41 PM	1	NE and Beach Drive NE (Catch Basin at Dash Point Park	J101426	676	754	93	[South of Flow Hydrant] Intersection of Soundview Drive	J59698	4721	85	63	91.9	84.0	62.4	22.0	21.6	-0.4	-0.6	1.1	1.0
	226				12:43 PM	2	(Catch Basin at Dash Point Park entrance)			954		NE and Markham Road				53			52.9	32.0	31.1	-0.9			
	411				8:50 AM	1	Approx. 5134 SW 324th Street			754		[West of Flow Hydrant]				54			55.1	4.0	3.2	-0.8			
137	411	Twin Lakes	10	10/31/2017	8:52 AM	3	(Catch Basin)	J64554	5048	1,324	58	On SW 324th Place in front of Dash Point Highlands Park	J64564	5055	58	48	59.9	58.3	51.3	10.0	7.0	-3.0	-1.9	-1.9	-0.3
						0				-														_	
138	411	Twin Lakes	10	10/31/2017	9:44 AM	1	Approx. 4520 SW 316th Place (South side of road in ivy - Catch	J103556	5008	908	84	[Southeast of Flow Hydrant] Just north of intersection of 45th	J66062	5007	72	61	85.6	72.7	65.6	11.0	7.1	-3.9	-2.6	-1.6	-0.7
	411		-		9:46 AM	2	Basin)			1,468		Place SW and SW 317th Place				47			49.0	25.0	23.7	-1.3			
	411				9:17 AM	1	Intersection of SW 324th Street			969		Masthurset of Flore United 2				77			78.6	7.0	6.6	-0.4			
139	411	Twin Lakes	10	10/31/2017	9:18 AM	2	and SW 321st Street	J103034	4956	1,518	85	[Northwest of Flow Hydrant] Approx. 4014 SW 321st Street	J103036	4955	84	70	88.2	85.2	71.5	14.0	13.8	-0.3	-0.3	-3.2	-1.2
						2	(Catch Basin)			-						70				-		-0.3			
140	411	Twin Lakes	10	10/31/2017	10:47 AM	1	Intersection of 39th Ave SW/SW 314th Street and 39th Court SW	J65658	4934	754	55	[Northwest of Flow Hydrant] Intersection of SW 314th Street	J65712	9544	52	38	57.3	53.9	43.8	14.0	10.2	-3.9	-0.3	-2.3	-1.9
140	411	I WIII LANCS	10	.0/01/2017	10:49 AM	2	(Catch Basin)	000000		1,212	33	and 40th Avenue SW	003/12	5544	JE	33	57.5	33.5	31.7	19.0	22.2	3.2	0.0	2.0	1.5
	346				12:18 PM	1	[Northwest of Flow Hydrant]			773		Intersection of SW Dash Point				61			58.6	2.7	3.8	1.1			
141		NE Tacoma	10	10/31/2017			Approx. 4830 SW 310th Street	J66202	5039		64	Road and 48th Ave SW	J99792	5080	64		62.7	62.4					1.0	1.3	1.3
	346				12:20 PM	2	(Ditch)			1,167		(Flow into swale/woods to east)				57			54.8	6.7	7.7	0.9			1
	346				11:54 AM	1	Intersection of SW 314th Place			754		[South of Flow Hydrant]	105			41			39.0	6.0	6.1	0.1			
142	346	NE Tacoma	10	10/31/2017	11:57 AM	2	and 51st Avenue SW (Catch Basin)	J65952	9809	1,237	55	South end of 51st Avenue SW at dead end	J65956	5049	47	32	53.1	45.1	30.6	15.0	14.5	-0.5	-0.2	1.9	1.9
	0.0					-	(Galon Dabili)			.,207		0000 010	L	1		52			00.0	.5.0		0.0			

Tacoma Water
Hydraulic Model Calibration Data
NE Tacoma Operating Area (Not Including 549 NE Tacoma Zone)

							Hydrant Flow Tested					Hydrant for Pressure Measurements													
							Field Measurements				Field Measurements					Model Results			Field Diff Model Diff					Static P	
						Duration		F1 Model		Flow	Static		R1 Model		R1 Static	R1 Residual	F1 Static	R1 Static	R1 Residual	Pressure	Pressure	Error	Error	Diff	Diff
Tes		ressure Zone				of Test		Node	Hydrant	Calc	Pressure		Node	Hydrant		Pressure	Pressure	Pressure	Pressure	F1	F1	(per flow)	(per site)	(Flow Hyd)	(Static Hyd)
No	HGL (feet)	Descriptor	Figure No.	Date	Time	(mins)	Location	No.	No.	(gpm)	(psi)	Location	No.	No.	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)	(psi)
14	346	NE Tacoma	10	10/31/2017	12:42 PM		On 37th Place SW just south of intersection with SW 309th Street	J99876	657	1,012	89	[Northeast of Flow Hydrant] Intersection of SW 309th Street	J99864	658	79	63	88.8	80.4	66.9	15.5	13.5	-2.0	0.7	0.2	-1.9
	346				12:44 PM	1	(Catch Basin)			1,580		and 36th Court SW				45			43.4	33.5	37.0	3.5			
14	186 186	Lakota Beach	10	10/31/2017	1:11 PM 	1	Intersection of SW 304th Street and 33rd Ave SW (Only flow 1 port and direct flow to south into woods)	J100340	4872	754	66	[Northeast of Flow Hydrant] Intersection of 38th Ave SW and SW 302nd Place	J66510	4859	60	48 	65.6	58.3	45.9	12.0	12.4	0.4	0.4	0.4	1.7